

State of Idaho

DEPARTMENT OF WATER RESOURCES

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> C. L. "BUTCH" OTTER Governor DAVID R. TUTHILL, JR. Director

November 27, 2007

Ms. Debbie Suehr Bureau of Reclamation 1150 N. Curtis Road Boise, ID 83706

RE: Request under Freedom of Information Act

Dear Debbie:

The Idaho Department of Water Resources requests copies of the following information under the Freedom of Information Act (FOIA):

Minidoka Project, ID-WY, North Side Pumping Division - DPR appendix

- Project Lands (June 1952; rev. July 1956) no.1
- Supplement to Project Lands (1956)
- Water Supply Supplemental (1954); no. 1
- Supplemental Drainage (1954); no. 1

Minidoka Project, ID-WY, North Side Pumping Division, Definite Plan Report, Volume 1, General Plan, February 1955, Region 1

Three revision sheets inside

Minidoka Project, ID-WY, North Side Pumping Extension – PFWD appendix

Drainage Investigations, Supporting Computations (December 1974)

Minidoka Project, ID-WY, North Side Pumping Division Extension – Planning Report/Draft EIS

Hydrology (July 1985); no. 1 [include large drawings/maps full size]

Minidoka Project, ID-WY, North Side Pumping Division, Drainwater Management Plan, <u>Draft Environmental Assessment</u>, December 1993, PN Region

Minidoka Project, ID, WY, North Side, *Minidoka North Side Resource Management Plan*, January 2005, SRAO (CD version)

If you have any questions, please call me at (208) 287-4841.

Minidoka Project, Idaho-Wyoming North Side Pumping Division Extension

HYDROLOGY APPENDIX

July 1985

Copy No. 1

Minidoka Project, Idaho-Wyoming North Side Pumping Division Extension

HYDROLOGY APPENDIX

Bureau of Reclamation Boise, Idaho July 1985

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HYDROLOGY APPENDIX

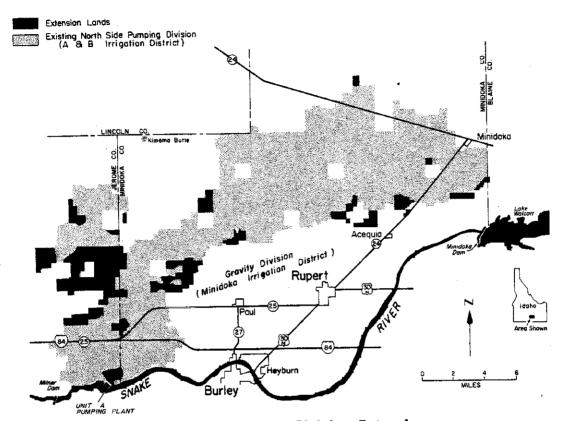
INTRODUCTION

The Minidoka North Side Pumping Division Extension project study is being conducted under the authority of Public Law 92-199, December 15, 1971. The North Side Pumping Division Extension of the Minidoka Project is located on the southern portion of the Snake River Plain and occupies a slightly elevated belt of land to the north and west of the Gravity Division (part of the Minidoka Project) in Minidoka and Jerome Counties (see North Side Pumping Division Extension map). The surface in the general study area is flat to gently rolling, with smooth benches and some small knobs. The elevation of most of the Extension lands are from 4200 to 4300 feet.

The study area is underlain by the Snake Plain aquifer, one of the most productive ground-water aquifers in the world (see Upper Snake River Basin map). The study area is located on the lower portion of the aquifer, which benefits from recharge resulting from surface water diverted to irrigate thousands of acres of lands upstream. Most of the discharge from the lower portion of the aquifer occurs downstream from the study area in the general area of Thousand Springs; more than 4 million acre-feet is discharged in that reach annually.

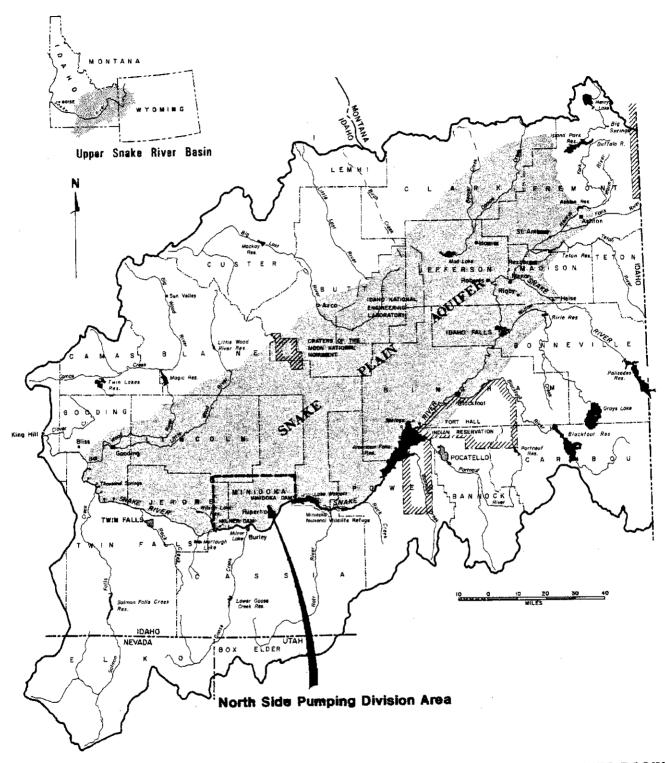
The potential Extension includes about 13,410 acres of dry Federal land under Reclamation withdrawal and about 3,750 acres of Bureau of Land Management land. For convenience, both the Reclamation withdrawn lands and the Bureau of Land Management lands are referred to as the Extension land in this study.

The Recommended Plan proposes to provide a full water supply to about 10,210 acres of land scattered throughout the North Side Pumping Division. This includes 9,400 acres of Extension land and 810 acres to receive a



North Side Pumping Division Extension

DEC 1983



UPPER SNAKE RIVER BASIN

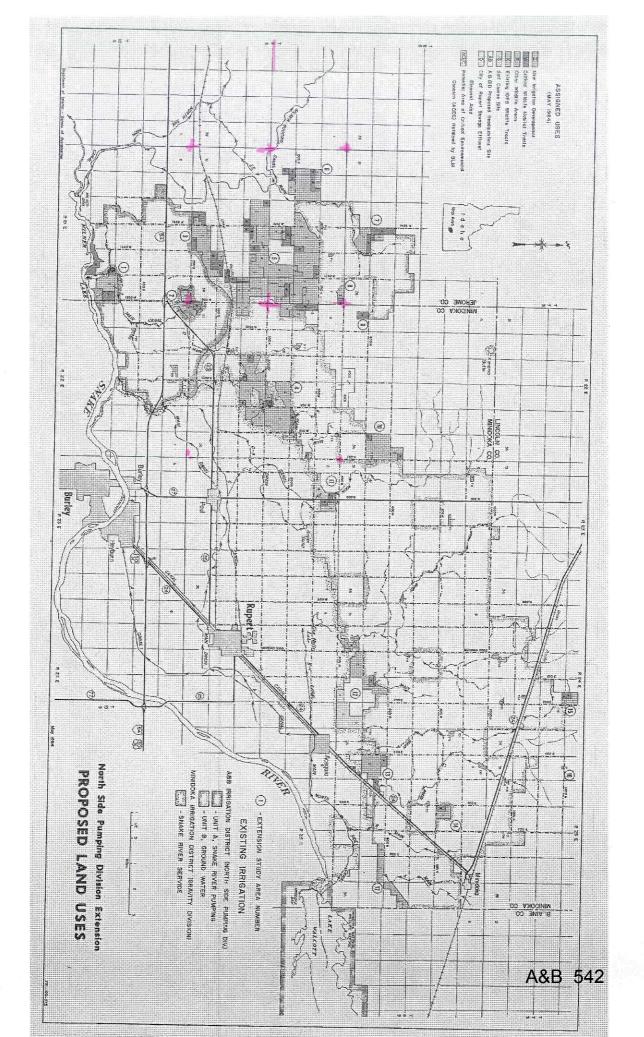
replacement water supply. The lands are located in 14 separate tracts (see Proposed Land Uses map) and range in size from 10 to more than 5,000 acres. Some of this land was considered for development with the rest of the original North Side Pumping Division, but funding limitations prevented its inclusion in the project at that time. Under the Recommended Plan, tracts 1, 2, 3, and 4 will be served from surface water; tracts 5, 6, 7, 10, 13, 14, 15, and 17 will be served from ground water; and tracts 11 and 12 will be served by a combination of drainage and ground waters. The 810 acres to receive a replacement supply are located in Area 4 and would be irrigated with surface water. Presently, they are irrigated with ground water.

The study also evaluates 29 existing tracts of Federal lands which were set aside in conjunction with the development of the North Side Pumping Division for wildlife management by the Idaho Department of Fish and Game. These tracts, which are located throughout the division, include a total of about 1,020 acres.

The climate of the study area is similar to that of the rest of the southern portion of the Snake River Plain. Precipitation is low, averaging only about 10 inches annually. The growing season precipitation is unpredictable and averages only about 4 inches a year. Half the 4-inch total may be received in 1 month, and little or no rain may fall in other months. Summer temperatures are high, and winters are cold. The growing season for hardy agricultural crops averages 190 days, and the frost-free period averages 130 days.

EXISTING NORTH SIDE PUMPING DIVISION

The North Side Pumping Division was constructed in the 1950's and irrigates 76,800 acres of land. Surface flows of the Snake River and ground water from the Snake Plain aquifer constitute the sources of irrigation water A&B 541



to the project. Unit A is comprised of 14,600 acres of irrigated land and is served from surface water pumped from the back waters of Milner Dam. Ground-water pumping from the Snake Plain aquifer irrigates 62,200 acres in Unit B. The A&B Irrigation District operates and manages the project.

A portion of the water supply for Unit A is provided by storage in American Falls and Palisades Reservoirs. Water for Unit A is pumped from the Snake River by a pumping plant 8 miles west of Burley. The pumping plant delivers water to a canal system serving the Unit A lands.

A total of 177 deep wells provide the ground-water supply for Unit B. The wells are 12 to 24 inches in diameter, average pumping head is about 200 feet, and the average well discharge is about 6.5 cubic feet per second (ft³/s). Unit B was the first large-scale ground-water pumping project on the Snake Plain. Since it was built, large acreages have been irrigated from ground water elsewhere on the plain.

A distribution system consisting mainly of unlined ditches was originally used to distribute water in both units. In recent years, many irrigators have converted to sprinkler systems.

The project area lacks a well-defined stream drainage pattern because of its youthful stage of geologic development. As a result, the project area has some enclosed drainage basins--relatively shallow depressions with no natural drainage outlets. Project drainage for runoff and return flows in those areas has been accomplished by the construction of drainage or injection wells which are drilled into highly porous zones of the underlying lava rock.

The irrigation of some of the high quality Extension lands is needed to expand the economic base of the area. Based on public input, plus economic, operational, and social needs of the present farm operators, it has been concluded that the most pressing agricultural need is the addition of small or

moderate amounts of land to existing farms rather than the establishment of new farm units.

Many of the existing farms in the North Side Pumping Division are quite small by present-day standards; average ownership in the A&B Irrigation District is about 135 acres. Because of the small farm sizes, some of the farms in the area are marginal or may be losing money under present conditions. On those farms, the addition of a relatively small amount of new land could mean the difference between economic survival and failure. Further, the layouts of some of the existing farm units do not lend themselves to improved farming methods, such as conversion from gravity irrigation to the more efficient types of sprinkler irrigation. In a number of cases, the acquisition of a relatively small acreage of Extension land could permit the conversion to more efficient farm practices.

CHAPTER 1 -- WATER RESOURCES

The water supply can be categorized by the source--surface water or ground water. To avoid confusion, the water supply discussion was divided into two sections--surface water and ground water. The surface-water discussion follows, and the lands served from ground water will be discussed in the following chapter entitled "Ground-water Resources."

Surface-water Supply--North Side Pumping Division

Sources of surface water available to the North Side Pumping Division include Snake River flows, Snake River storage, the "rental pool," and surface drainage water. The United States holds a Snake River natural flow right of 267 ft³/s (priority date April 1, 1939) in trust for the A&B Irrigation District. In addition, the district has secured storage water in two of the

seven Reclamation upper Snake River reservoirs. The priority dates and storage space in the two reservoirs are shown in table 1.

Table 1.--Priority Dates and Storage Space

Reservoir	Priority Date	Storage Space
		acre-feet
American Falls	3/30/21	$46,826\frac{1}{}$
Palisades	7/28/39	90,800
Total storage space		137,626

^{1/} Reduced from 47,593 acre-feet in 1978 due to sediment deposit

During the 1973 through 1977 period, American Falls storage was restricted to 66.2 percent of capacity due to dam safety considerations and reconstruction of the facility. Accordingly, the district's storage capacity was limited to 31,495 acre-feet during this 5-year period. Further, the district's American Falls storage right was 47,593 acre-feet prior to 1973; however, subsequent to reconstruction (1978), each shareholder's capacity was revised to reflect the reduction in American Falls capacity due to sediment deposits.

Surface Water--Snake River

The irrigated area served from the Snake River diversions and its tributaries at and above Milner Dam exceed 1 million acres. Diversions to these lands range from 4 acre-feet per acre or less on pumping projects to 15 acre-feet per acre in areas where subirrigation is practiced. A substantial amount of the diverted water returns to the river, however, and is reused during the same season or is stored in American Falls Reservoir during the winter for irrigation use during the following season.

Historic records indicate the surface-water resources serving the irrigated area above Milner Dam are highly developed. Under present development, the Snake River flow would have been fully controlled and utilized for irrigation except releases required to meet established water rights below Milner Dam during the drought period from 1931 to 1935. The opportunities for expanding the lands irrigated from this source are limited without additional storage in the system.

During periods of below average runoff, the natural flow in the Snake River has been entirely appropriated. During these periods, the district's surface-water right priority date does not allow them to divert natural flow to Unit A. However, in periods of average to above average runoff the district diverts natural flow to Unit A, and records maintained by the district indicate that diversions during these periods can meet all of the Unit A irrigation demands through June and, in very wet periods, the early part of July. The use of natural flow whenever available insures the project's carryover storage will be at a maximum level when entering a dry cycle.

The irrigation of Unit A land with Snake River water as a source of supply is dependent on adequate storage water. To insure a base supply during the dry cycles (regardless of length), the reservoir providing the storage water must have a storage right which guarantees a firm yield (one to one), which American Falls Reservoir provides. Palisades Reservoir, on the other hand, does not have a one-to-one yield; however, it was designed to capture surplus flows during wet cycles and carry storage over for irrigation use during dry cycles.

The Bureau of Reclamation began development of the North Side Pumping Division facilities in the early 1950's, and the project was essentially

complete in the early 1960's. In 1963, the district began and has maintained diversion records for the Unit A Snake River pumping station. For the 1963 through 1983 period, the 21-year average annual surface water diverted to Unit A was 56,400 acre-feet, of which 23,000 acre-feet was natural flow and 33,400 acre-feet was storage. The records indicate Unit A lands have not experienced an irrigation shortage for the 21-year period examined, and there were only 3 years (1966, 1973, and 1977) in which the irrigation diversions exceeded natural flow plus the available storage in American Falls and required drafts from Palisades storage. Two of three years occurred during the 5-year period in which American Falls storage was restricted to 66.2 percent and the district's storage in American Falls was limited to 31,495 acre-feet.

When a spaceholder utilizes Palisades storage, a conveyance loss is charged to the user. The quantity of storage water released from Palisades Reservoir includes this conveyance loss. The district's average annual storage water diverted to Unit A was 33,400 acre-feet; however, because they utilized Palisades storage in 3 of the 21 years examined, the average annual storage water charged (released) was 33,500 acre-feet. In 1978, this practice of charging a conveyance loss to Palisades storage was discontinued.

Storage from Palisades Reservoir (including conveyance losses) required to meet the irrigation requirements of the Unit A lands in 1966, 1973, and 1977 were 8,690, 17,130, and 24,150 acre-feet, respectively. With American Falls Reservoir having a firm yield (one to one), it can be assumed that the Palisades storage requirements in 1973 and 1977 would have been significantly reduced had the storage restrictions on American Falls Reservoir not been in force. Assuming no storage restrictions on American Falls, the estimated

Palisades storage requirements (including conveyance losses) in 1973 and 1977 would have been about 1,800 and 8,820 acre-feet, respectively.

At the beginning of each irrigation season, storage space in the upper Snake River reservoirs is allocated to each spaceholder. During the 1963 through 1983 period, the 21-year average annual storage allocated to the district was 129,400 acre-feet (43,500 acre-feet from American Falls and 85,900 acre-feet from Palisades). As previously discussed, the average annual storage charged to the district over this same period was only 33,550 acre-feet (31,170 acre-feet from American Falls and 2,380 acre-feet from Palisades. Had American Falls storage not been restricted to 31,495 acre-feet (about a 15,330-acre-foot reduction) from 1973 through 1977, the average annual American Falls allocation would have been about 47,200 acre-feet. Further, the average annual storage required from Palisades Reservoir would have been reduced from 2,380 acre-feet to about 900 acre-feet (about 1 percent of the average annual Palisades storage allocated).

The district has maximized their natural flow diversions, and when storage has been used to meet the irrigation requirements of the Unit A lands, the district has utilized American Falls storage first and Palisades storage second. This practice has maximized annual carryover storage because the district receives their full American Falls storage allocation about 98 percent (estimated) of the time, and when storage has been required, the American Falls storage is capable of meeting the majority of the annual Unit A diversion requirements. In the 3 years Unit A storage requirements exceeded the available American Falls storage, between 84 and 96 percent of the annual storage requirements would have been provided by American Falls assuming no storage restrictions during the 1973 through 1977 period.

Under the proposed project, 3,950 acres would be provided a full water supply in Unit A, and a portion of the annual carryover storage would be utilized to meet the irrigation requirements of these new lands. Examining the district records and assuming the new lands would utilize available American Falls storage first, the estimated 21-year average annual available American Falls storage to the proposed new lands was 15,100 acre-feet, of which an average of approximately 10,000 acre-feet would be utilized. Storage requirements for the new lands not satisfied by American Falls storage would be met from Palisades storage. Based on the historic records, the estimated 21-year average annual Palisades storage requirements for the new Unit A lands would be less than 2,500 acre-feet. The estimated 21-year average annual Palisades storage requirements for the existing lands plus the proposed new lands would represent about 5 percent of the 21-year average annual Palisades storage allocation. The district's records indicate it is possible to serve the proposed new Unit A lands with available storage from American Falls and Palisades Reservoirs and still maintain adequate carryover storage to assure a dependable water supply during dry cycles to all (existing and new) Unit A lands.

There is one additional potential source of Snake River water which has not been addressed—the rental pool. Rental pool water is made available by spaceholders when they determine storage water allocated to them is in excess of their needs. This water is offered to the rental pool by the spaceholder, at which time the "rental pool committee" determines if there is a need for the water offered and further who might use it. Individuals or canal companies who forecast their need for water greater than their ownership rights may apply for additional water from the rental pool committee on an annual or long-term (through lease agreements) basis. Based upon the decision

of the committee (which is comprised of representatives from the "Committee of Nine," Reclamation's Minidoka Project office, and the watermaster), the water is made available to the interested applicant for a fee. The fee in 1984 was \$2.50 per acre-foot of water.

Assuming the new irrigation demands require the A&B Irrigation District to utilize the rental pool, the quantity that might be used would not adversely affect the other users of the pool. There are no priorities for use of the rental pool, only the annual determination by the committee. The cost of water from the rental pool would be covered with the annual operation and maintenance charges made to the project. Based on the historical records, it appears highly unlikely this potential source of water will be required or utilized under the Recommended Plan.

Surface Water--Drainage

The proposed plan would develop a partial solution to the injection well problem by irrigating Extension lands from existing North Side Pumping Division drains wherever possible. There is a potential of using drainage water to satisfy part of the irrigation water requirements of several blocks of new land located near the terminus of major drainage ditches in the A&B Irrigation District Beginning in 1980, Reclamation, in conjunction with the A&B Irrigation District, began monitoring surface drainage flow during the irrigation season. In each year subsequent to 1980, several additional new measuring sites were established on injection wells and drainage ditches leaving the project.

The available records at each site vary from 1 to 4 years and indicate that the majority of the drains have periods during the irrigation season of minimal (zero) flow. There are substantial fluctuations in the daily discharge, and, to maximize the use of the drain flows, small regulating

facilities capable of storing the daily irrigation water requirements of the new lands would be required on the majority of the drains. On several of the drains, the size of the regulating facility required to maximize drainwater use would inundate an area of existing farmland which is approximately the same size as the area of new land proposed for development. Further, the annual yield on the majority of the drains will not meet the annual irrigation water requirements of the proposed new lands.

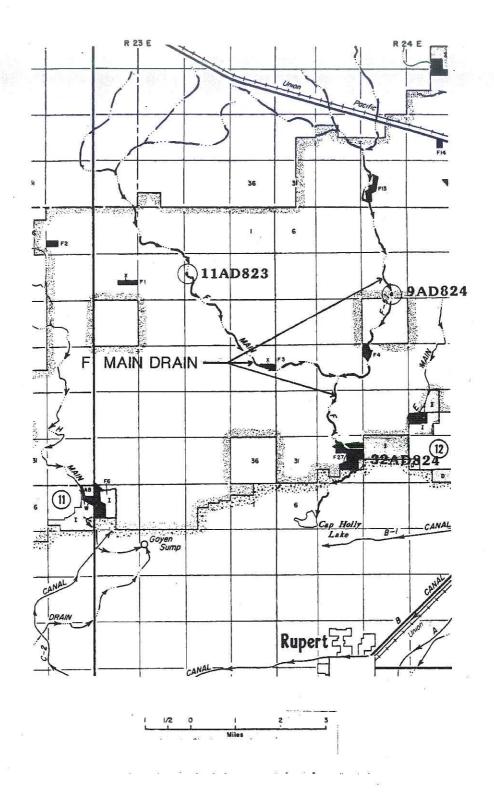
There are two areas where use of drainage water was found feasible as a potential source of water--tracts 11 and 12 (see Proposed Land Uses map). Drainage water will be diverted to Area 11 from the H Main Drain, and Area 12 will utilize drainage water from the E and F Main Drains. Records for the H Main Drain indicate this drain is capable of providing part of the irrigation water requirements of Area 11 (120 acres), and the records for the E and F Main Drains indicate there generally is sufficient drainwater to provide a dependable water supply to the land located in Area 12 (460 acres); however, some supplemental pumping from the existing project wells may be required in the early spring and late fall.

In 1982, a data collection site was established on the H Main Drain. There are two injection wells (4BD923 and 5AD823) on this drain, and the collection site was established between the two wells. In 1983, the collection site was relocated upstream of both injection wells so the total available drainwater could be estimated. The limited data available at the two sites indicate the H Main Drain can provide part of the irrigation water requirements to Area 11; however, a new ground-water well will be required to augment the drain flows for much of the irrigation season. Based on this limited data, the estimated drainage water which can be utilized by Area 11 is about 100 acre-feet per year (average).

Currently, about 70 acres in Area 12 are leased from the Government and are irrigated from drainwater in the E Main Drain. The estimated drainage water diverted from the E Main Drain is about 200 acre-feet per year (average), and it was assumed this practice would continue when the project is developed. The remaining 390 acres in Area 12 would be served from the F Main Drain.

Data collection on the F Main Drain began in 1981. There are three injection wells on this drain (11AD823, 9AD824, and 32AD824), and in the 1981 and 1982 seasons, the data collection site was located below all three wells. The total amount of water in the F Main Drain could not be quantified during the 1981 and 1982 seasons because the volume of water entering the three wells was not recorded.

In 1983, the collection site was relocated to just upstream of drain well 32AD823, and a new collection site was established to monitor the flows entering injection well 9AD824. The map of F Main Drain on the following page shows the location of the three injection wells on F Main Drain, noting the position of the recorder sites (above 32AD824 and at 9AD824) still does not account for the water entering injection well 11AD823. The 1983 data collected at the site just above drain well 32AD824 indicates there is sufficient drainwater in F Main Drain to provide an adequate water supply for 390 acres in Area 12 from mid-June through the end of August and would only require a very small regulating facility. In the period prior to mid-June and subsequent to August, the water entering the injection wells 11AD823 and 9AD824 would be diverted to Area 12 if water shortages occur. In the event the new lands would experience a shortage in the early spring to late fall, the existing project wells could be utilized to insure an adequate water supply for this period. The limited data available indicate the estimated



F MAIN DRAIN

- INJECTION WELL
- 1 -EXTION STUDY AREA NUMBER

EXISTING IRRIGATION

A&B IRRIGATION DISTRICT (NORTH SIDE PUMPING DIV.)

- UNIT B. GROUND WATER 553
MINIDOKA IRRIGATION DISTRICT (GRAVITY DIVISION)

- SNAKE RIVER SERVICE

drainage water which would be diverted from the F Main Drain to Area 12 would be about 900 acre-feet per year (average).

CHAPTER 2--GROUND-WATER RESOURCES

Introduction

A primary purpose of this chapter is presenting geohydrological data to aid planners in irrigation well-site selection and construction design for the North Side Pumping Division Extension studies.

Ground-water pumping for irrigation began in southern Minidoka County in 1947. The Bureau of Reclamation drilled three irrigation wells in 1948 and leased them to private landowners for irrigation. These wells proved the feasibility of ground-water pumping for irrigation and were the forerunners of numerous irrigation wells in the area.

Unit B contains about 62,200 acres, irrigated with ground water pumped from 177 wells located throughout the project area. Records maintained by the district indicate the 20-year average (1963 through 1982) annual ground water pumped to Unit B lands was 208,240 acre-feet.

A water level decline, beginning in the 50's and thought at the time to be caused by ground-water pumping, resulted in about 90 wells being deepened. Deepening began in about 1958 or 1959, and continued until about 1964. A few wells were deepened twice. Several wells were deepened immediately after completion because of inadequate yield. One well was deepened in 1981.

The ground-water requirements for the proposed new Unit B lands will be provided from 18 ground-water wells (14 new and 4 existing) located near or within the individual tracts of land. During original development, five wells were drilled and not used. One well, 33A824, has been sold to the city of Rupert. The remaining four wells would be used for a water supply. Some or A&B 554

all of these wells may require some renovation to bring them up to current Reclamation well construction standards.

Conclusions

Net ground-water pumping for the proposed North Side Pumping Division Extension project is expected to increase total pumpage from the area by approximately 13,520 acre-feet per year--about a 2.5-percent increase over present use in the Snake Plain aquifer in the Rupert to Thousand Springs area and about a 1-percent increase for the entire Snake Plain aquifer. The plan would provide for 15,620 acre-feet of additional pumpage but would be offset by 2,100 acre-feet for the replacement area (810 acres).

The major influence upon ground-water level declines and recoveries is climate; therefore, the increased pumpage is not expected to have a measurable effect upon present water levels. Locally, a water level drop of 1 to a few feet may occur but, because of the enormity of the Snake Plain aquifer and the ground-water supply, the effect will be difficult to identify at any distance beyond the project area. The effect of the additional pumpage, a possible 0.25-percent decrease in aquifer discharge at Thousand Springs, will not be identifiable at the springs.

Geology

The project area topography consists of gently rolling hills typical of a basalt flow landscape. This is in contrast to the relatively flat river formed landscape to the south. An abrupt rise in surface elevation marks the southerly extent of lava flows. This topography change is the dividing line between the Minidoka Irrigation District to the south and the North Side Pumping Division Irrigation District on the north.

A blanket of wind-deposited silt, sandy clay, and very fine sand from a few inches to a few tens of feet thick covers most of the underlying basalt. There are, however, a few isolated basalt outcrops dotting the area. In other areas, basalt debris at the surface testifies of more basalt at shallow depths.

The basalt is typical of Snake River basalt being made up of everything from ash, cinders, and highly fractured rock to thick, dense, and massive flows. The sediment is mostly clay and silt with some coarse sand and little gravel.

The subsurface geology of the Minidoka Irrigation District is comprised predominantly of sediment with the amount of sediment decreasing east, west, and north. Nearly all the area beneath the North Side Pumping Division Unit B is made up of basalt with few to minor amounts of sediment. The basalt is mostly highly to moderately broken with many innerflow zones of rubble and/or cinders. The subsurface beneath tract 4 is composed of basalt innerbedded with substantial amounts of mostly fine-grained sediment. The subsurface geology of the area occupied by North Side Pumping Division Unit A is also primarily basalt with minor amounts of sediment locally.

Geohydrology

Snake Plain Aquifer

The ground-water source for the North Side Pumping Division is the extensive Snake Plain aquifer; hence recharge and/or withdrawal changes within the Snake Plain aquifer affects the North Side Pumping Division aquifer, and recharge and/or withdrawal changes within the North Side Pumping Division aquifer affect the Snake Plain aquifer, primarily aquifer users down gradient from the North Side Pumping Division. Therefore, a look at the Snake Plain aquifer is essential to understanding the ground-water regime of the North

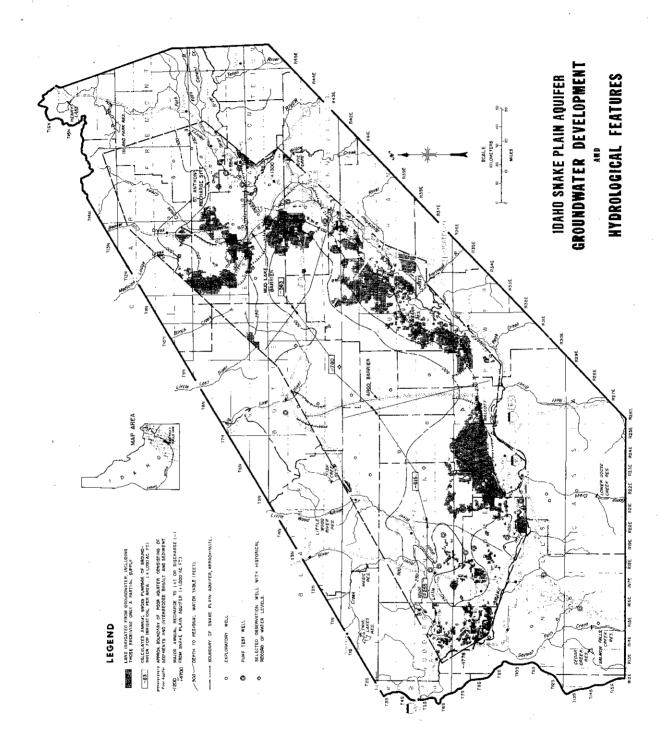
Side Pumping Division. The following is a discussion of the Snake Plain aquifer from "Investigations Ground-Water Supply for Salmon Falls."

The Idaho Snake Plain aquifer (see map on following page) is one of the largest and most prolific in the Nation. The aquifer extends from St. Anthony to Bliss, a distance of 180 miles, and averages 60 miles wide. Area extent is about 10,800 square miles or 6,900,000 acres.

The trough which contains the Snake Plain aquifer is a broad downwarp or downdropped fault block extending over 190 miles from Bliss to Ashton. Rocks which form the bordering mountains and the floor of the trough are largely consolidated materials of low permeability. The trough is filled with a thick sequence of lava flows including basalts of the Snake River Group. Major sites of outpouring of lava appear to have been concentrated in three areas of the plain: An area north of Rupert; on the plain between Blackfoot and Arco; and an area south of Dubois. There is evidence that volcanic activity in the three areas was progressively younger from west to east. The concentrations of lava accumulations tend to be encircled by sediments.

As the trough filled, the ancestral counterparts of present-day streams were repeatedly diverted, confined, and ponded. The successive layers of basalt in areas of volcanic activity continuously maintained higher elevations and stream courses were restricted to lower topography around the margins of the accumulating lava flows. Sediments from ancient streams and lakes accumulated to great depth in approximately the same location as present streams.

The Idaho Snake Plain Aquifer map shows a boundary between areas of predominantly basalt and areas of alluvium, in part interfingered with basalt. In most areas of surface or near-surface sediments such as at Rupert, the upper end of Walcott Reservoir, American Falls Reservoir, and at Mud Lake, the



deepest drilling of about 1,000 feet is still in alluvium. Several deep wells in the Mud Lake area penetrate varied clays at about 800-feet below surface showing that a lake existed in that area in glacial time. Most of the sediments are fine grained and poorly permeable although there are occurrences of sand and gravel.

The occurrence of poorly permeable lake sediments such as at Mud Lake, extending across the aquifer, in effect form a "dam" which segments the aquifer. The dam formed by the Mud Lake sediments is referred to as the Mud Lake barrier. The water table above the barrier is at about elevation 4700 and drops to 4550 immediately below. A similar zone of steepened gradient extends across the plain between Arco and Lake Walcott (Crosthwaite, 1973). This is interpreted as a barrier of sediments similar to the Mud Lake barrier and is noted as the Arco barrier on the Snake Plain Aquifer map. The water table above the barrier is at elevation 4400 and descends to elevation 4100 below the barrier in a distance of 3 miles.

Although the Snake Plain aquifer appears to be segmented into three separate compartments or reservoirs, there is a degree of hydraulic continuity. The effect of the aquifer dams is not fully understood at this time but may be significant in regard to location and effects of recharge or withdrawals from the aquifer. Water table changes occurring in one segment appear to be dampened in passing through a downgradient barrier.

Thickness of the Snake Plain aquifer is unknown except in marginal areas of the plain. The deepest drilling on the plain is about 10,000 feet. Present interpretation of a geophysical profile developed by the Geological Survey between Blackfoot and Arco indicates a maximum thickness of 7,000 feet. There are indications that permeability may decrease with depth so that the maximum effective aquifer thickness may be considerably less than 7,000 feet.

Transmissivity $\frac{1}{2}$ of the aquifer ranges upward to 36 x 10^6 gpd/ft as measured in controlled pump tests. Values approaching 100×10^6 gpd/ft are suspected under some interior portions of the plain to accommodate the known magnitude of flow. In comparison, transmissivity values for the Columbia River basalts are in the range of 12,000 to 30,000 gpd/ft in eastern Washington, and 67,000 to 130,000 gpd/ft in the Umatilla, Oregon, area. At The Dalles, Oregon, transmissivity of the Yakima basalt ranges upward to about 300,000 gpd/ft.

A true value for storativity 2/ has not been accurately derived for basalt aquifers from relatively short-period pump tests. Most investigators have used a range of storage values for the Snake Plain aquifer of between 10 and 20 percent. Even using the smaller of the two, a change in water table of 1 foot across the plain represents a change of water in storage of 690,000 acre-feet.

The aquifer is fed by seepage from streams which enter or cross the plain, underflow from tributary valleys, irrigation return flows, and from precipitation on the plain and bordering foothills. Prior to the advent of large scale irrigation about 3 million acre-feet per year moved through the basalts in this way--presently recharge from surface-water irrigation amounts to about an additional 4.8 million acre-feet per year. Discharge from the aquifer occurs as spring flows concentrated near the upper end of the American

^{1/} Transmissivity indicates ability of aquifer to transmit water--gallons per day which would move through a 1-foot wide slice of the aquifer under unit gradient

^{2/} Storage or storativity is an indication of effective porosity of an aquifer-the volume of water an aquifer releases or acquires per unit surface area of the aquifer per unit change in head. At 10 percent storage, a water table rise or decline of 10-feet under 1 acre would equal a gain or loss of 1 acre-foot of water; and at 20 percent storage a gain or loss of 2 acre-feet.

Falls Reservoir and at Thousand Springs, and as ground-water pumpage for domestic, municipal, and irrigation supplies.

The Inflow, Outflow, and Storage map on the following page illustrates the Snake Plain aquifer water budget.

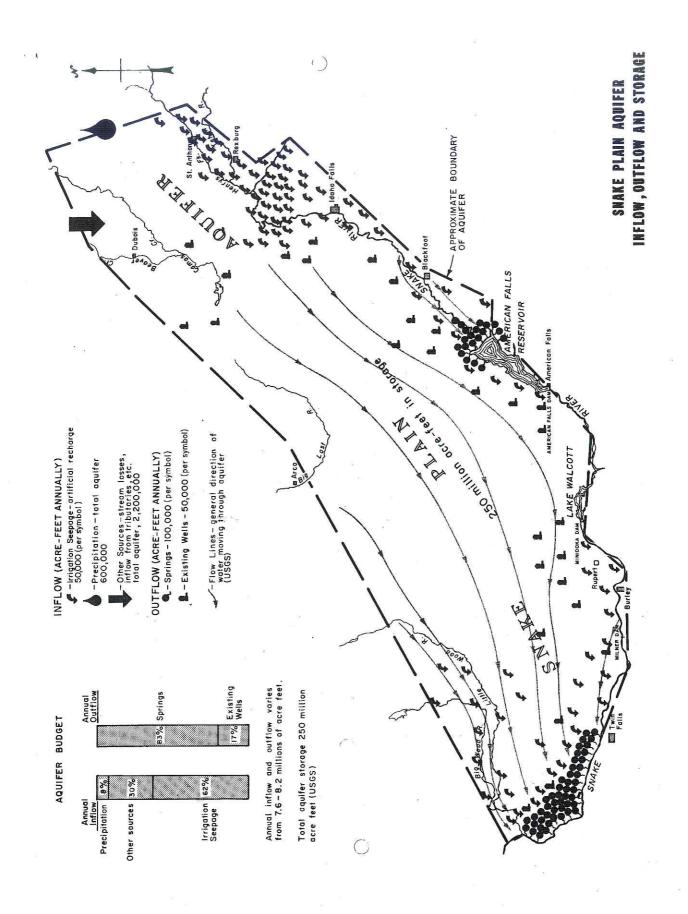
North Side Pumping Division Aquifer. -- The aquifer, as previously discussed, is made up of sediment and basalt. A few locations, particularly part of that area occupied by Minidoka Irrigation District, have wells that penetrate no basalt; other locations have wells with little or no sediment.

The sediment is mostly fine grained with some sand and little gravel.

Water yield from the sediment is generally small, up to a few tens of gallons per minute.

The basalt is made up of a series of thin flow sheets, from a few feet to several tens of feet thick. Where the flow sheets are deposited one upon another to form a relatively thick sequence, and where the basalt is highly fractured and/or contains numerous rubble or cinder zones, the water yield is large, up to several thousand gallons per minute. Where the flow sheets are made up of dense, and massive basalt and/or is covered, penetrated, or innerbedded with fine sediment, the water yield is small to moderate. One such area is in the southwest part of Unit B located mostly in T9S/R22E where several low yielding wells are found.

Here the aquifer is comprised of basalt innerbedded with substantial amounts of fine sediment. Some of the basalt in the upper part of the aquifer also contains fine sediment that reduces the permeability. The deeper basalt is relatively free of sediment, but must be thick, massive, and dense with a low permeability because water yield remains low despite more than 100 feet of exposed basalt aquifer in some wells.



The specific capacity of a well is a measure of the yield potential of that well and of the aquifer at that well site. Specific capacity is calculated by dividing the yield in gallons per minute by the water level drawdown in feet. The resultant value is, therefore, reported in gallons per minute per foot of drawdown, or, the aquifer should yield the calculated number of gallons of water each minute for each foot the water level is drawn down. The larger the specific capacity number the greater the yield potential for the well. The Specific Capacity map (map 1 at end of appendix) is a contoured illustration of the specific capacity values for the North Side Pumping Division area. The sediment portion of the aquifer penetrated by the wells was ignored because water yield from the sediment is small, probably not over a few gallons per minute. These specific capacities were derived from reported district well pump test data. The predominantly basalt aquifer has the highest specific capacity, sometimes as high as several thousand gallons per minute per foot of aquifer drawdown; whereas, the predominantly sediment aguifer has the lowest specific capacity, often as low as several gallons per minute per foot of drawdown.

The district well pump tests were run to determine whether the well would deliver the required yield for that site and were not run to determine hydrologic properties of the aquifer. Because of this, inspection of the Specific Capacity map will show a great diversity of specific capacity values throughout the irrigation district. Sometimes adjacent wells have greatly different specific capacity values, one high and the other low. The high value probably more nearly reflects the yield potential for the aquifer; therefore, the large value was plotted on the map.

A Depth to Water map (map 2 at end of appendix) was prepared from district measured water level data (spring 1981) and from surveyed well

elevations. The water table depth is about 80 feet below land surface on the southeast edge of the irrigation district and increases to about 400 feet on the west edge. The increasing depth results mostly from falling water table elevations east to west.

A Water Table Elevation map (map 3 at end of appendix) was drawn from spring 1981 district measured water levels and illustrates ground-water flow direction and ground-water recharge areas. Generally, the ground-water flow sweeps across the North Side Pumping Division area from northeast to west-southwest. Ground-water recharge from Raft River and Oakley Fan and especially from Burley Irrigation District and Minidoka Irrigation District areas flows in a west-northwest direction and is intercepted by the Snake Plain aquifer near the southern edge of the North Side Pumping Division area.

The water table elevation is about 4090 feet on the southeast edge of the North Side Pumping Division area and falls to about 3840 feet on the west edge of the district. This a drop of about 250 feet as the ground water crosses the district. The gradient is quite flat, less than 3 feet per mile, from the east edge of the district to a line running northerly between Burley and Rupert, then swinging in a westerly arc to north of Paul and finally trending northwesterly across the district (see Water Table Elevation map). West of the above line the gradient increases to about 15 feet per mile except for a circular flat area, with a gradient of about 1.5 feet per mile, covering about 25 or 30 square miles near Burley and Paul. The very flat area near Burley and Paul is probably the location of an almost all sediment aquifer.

Recharge

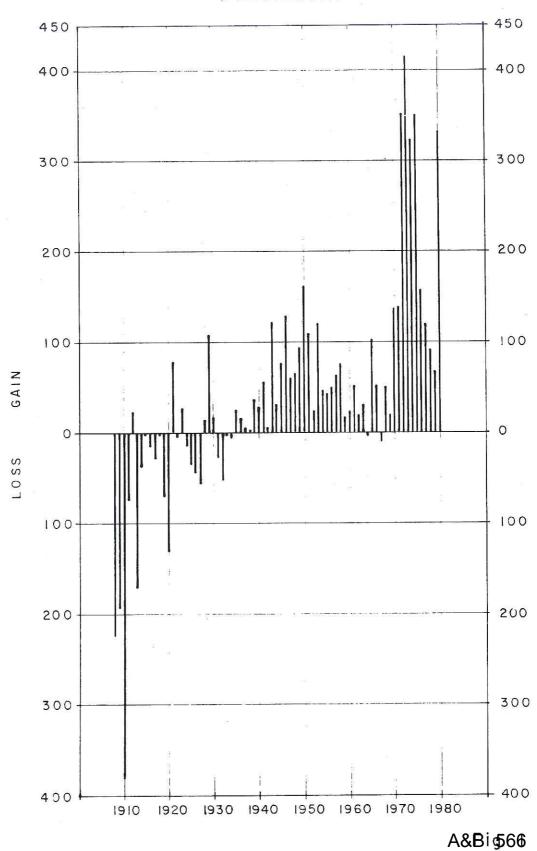
Because the aquifer beneath the North Side Pumping Division is part of the Snake Plain aquifer, most of its recharge is from that source. The three main recharge sources are precipitation, and surface-water losses from streams and irrigation. Precipitation amounts to about 10 percent of the recharge, loss from streams and other surface-water sources amounts to about 30 percent, and irrigation contributes about 60 percent. Secondary recharge sources, such as drain wells, originate locally.

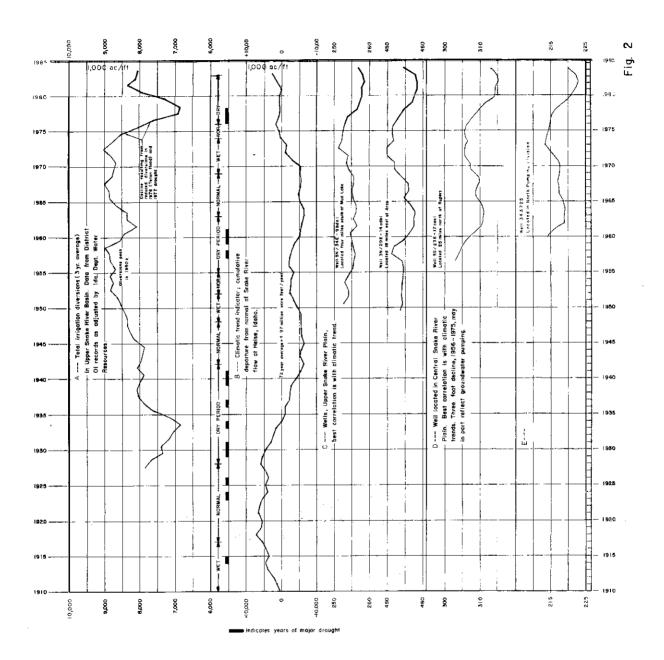
Snake River flow, loss or gain, between the Neeley Gage below American Falls Dam and the Minidoka Gage below Minidoka Dam has been computed for the period 1908 to 1980. Figure 1 is a plot of the loss-gain data. Losses were initially large, with a high in 1910 of about 381,000 acre-feet, and thereafter, decreased to a low in 1933 of about 1,000 acre-feet. The 1908 to 1933 period had 7 years of flow gain with a high in 1929 of about 108,000 acre-feet and a low in 1928 of about 14,000 acre-feet. Since 1935 there have been only 2 years of flow loss, 1964 with about 4,000 acre-feet and 1967 with about 24,000 acre-feet. During the 1935 to 1980 period there were large flow gains from 1972 to 1975 and in 1980 with a high of about 416,000 acre-feet and a low of about 324,000 acre-feet.

Hydrologists have been unable to positively identify the cause of the change from flow loss to flow gain. The effect of the flow change, however, is not apparent on the water level record. Much of the change occurred before North Side Pumping Division construction and during a period of increasing ground-water recharge from growing surface irrigation diversion throughout the Snake Plain aquifer area that peaked during the 1950's. See figure 2-A for a plot of surface-water diversions on the Snake Plain from 1928 to 1983.

Burley Irrigation District and Minidoka Irrigation District utilize Snake River water, diverted from Lake Walcott, as an irrigation supply. Both districts are ground-water recharge source areas for the Snake Plain aquifer beneath the North Side Pumping Division. The Burley Irrigation District is located south of the Snake River and the Minidoka Irrigation District is

SNAKE RIVER Computed Flow Gain or Loss Neeley Gage to Minidoka Gage X 1000 Acre Feet





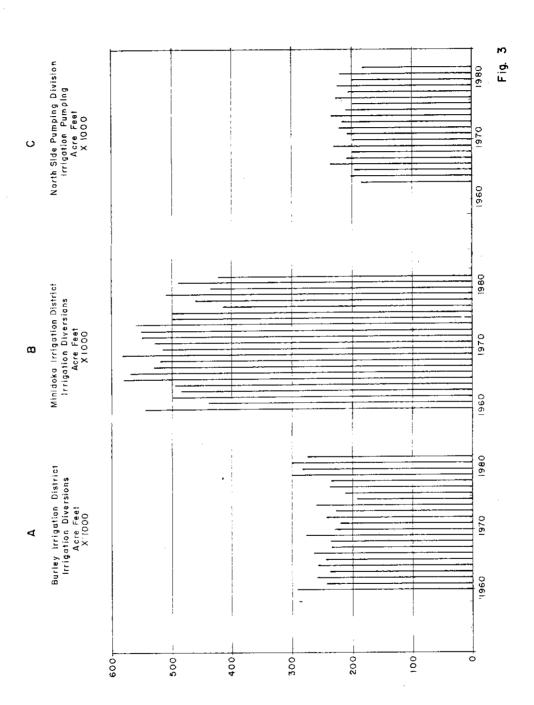
located north of the Burley Irrigation District between the Snake River and the North Side Pumping Division Irrigation District.

Figure 3-A and B are graphs illustrating Burley Irrigation District and Minidoka Irrigation District's irrigation diversions for a 22-year period from 1960 to 1982. The Burley Irrigation District irrigation diversions, figure 3-A, have remained fairly constant at about 240,000 acre-feet per year for the 22-year period. Whereas, the Minidoka Irrigation District irrigation diversion, figure 3-B, rose from 1960 to a high in 1965 and 1969 of about 580,000 acre-feet each year and then decreased to a low in 1977 of about 414,000 acre-feet and in 1982 of about 421,000 acre-feet.

The Minidoka Irrigation District irrigation diversions decrease as a result of a district water saving program practiced by the irrigators. This diversion decrease represents a sizable reduction in ground-water recharge for that portion of the Snake Plain aquifer feeding the North Side Pumping Division irrigation wells.

Due to the lack of data, an unknown amount of recharge to the aquifer comes from drain wells located throughout the project area. The amount of recharge is not large, probably not over a few thousand acre-feet per year. Discharge

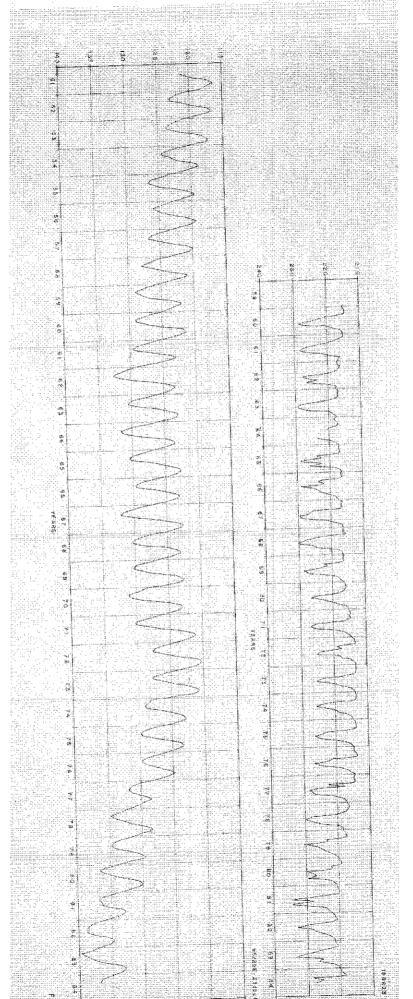
Discharge from the aquifer occurs as spring flow or from ground-water pumping. Ground-water pumping is the major aquifer discharge in the North Side Pumping Division area, with about 210,000 acre-feet pumped each year within the project. Figure 3-C illustrates district ground-water pumping from 1963 to 1982. Additional ground-water pumping of an estimated 400,000 acre-feet occurs in the area adjacent to and near the project, making this area one of the most intensely pumped on the Snake Plain.



Water Level Fluctuations.--Water level fluctuations are the aquifer's natural response to changes in storage. These fluctuations include: short-term (seasonal cycle) water table movement resulting from day-to-day changes in aquifer recharge or withdrawal over a period of a year; and long-term water table movement superimposed upon the short-term movement resulting from major changes in recharge or withdrawal extending over a period of several or more years. See figure 4 for illustration of long- and short-term water level fluctuation.

The short-term response is caused by changes in recharge from precipitation, river and reservoir losses, irrigation, and from withdrawals such as pumping. Short-term (yearly) fluctuations in the North Side Pumping Division range from about 4 feet in the porous basalt aquifer in the northern part of the project area to about 20 feet in the sediment, sediment-basalt aquifer in the southern and southwestern part of the project.

The long-term response is caused by such things as major precipitation changes over a period of several years, or by major surface irrigation and ground-water pumping changes. The most significant long-term change appears to be from precipitation. Comparison of the long-term hydrograph in the North Side Pumping Division area (figure 2-E, well 34A 725) with the Snake River flow hydrograph (figure 2-B) from the Heise gage show very close correlation. The Heise gauge was selected because it is upriver from major irrigation diversions and thus reflects changes in precipitation instead of changes in irrigation diversions and return flows. Well hydrographs located downstream from the Heise gage reflect a combination of influences from irrigation and precipitation. Therefore, similarities in the Heise River gage and downstream well hydrographs are caused by precipitation.



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The Water Level Change maps (maps 4 through 7 at end of appendix) are contoured plots of water level change within the North Side Pumping Division area. The long-term water level peaked in the early to mid-1950's and then began a water level decline period lasting until the early to mid-1960's. Because of the water-level decline (1950's to 1960's), ground-water geologist Jack Frink was hired to study the ground-water problem. During this decline period, there was about an average 12-foot aquifer water level drop. The 1953-58 to 1963 Water Level Change map (map 4 at end of appendix) illustrates declines ranging from 1 to 36 feet. In an effort to determine the cause of the decline, water-level hydrographs and other data from throughout the Snake Plain aquifer area were studied. The hydrographs included those within the North Side Pumping Division and other irrigated areas as well as those beyond direct influence of pumping or surface water irrigation. Mr. Frink estimated that 6 to 7 feet of the decline was probably caused by irrigation pumping and the remainder was caused by a drier climatic period.

A wetter climatic period from the mid-1960's to the early to mid-1970's, followed the 1950's to 1960's declining period and caused the water levels to recover an estimated 5 feet of the 12-foot decline. The 1963 to 1973 Water Level Change map (map 5 at end of appendix) shows recoveries ranging from 1 to about 15 feet. Another declining period began in the 1970's and continued until about 1982 (1973 to 1982 Water Level Changes map in pocket). There has been an estimated net 10- to 15-foot decline from the 1950's to 1982 (map 7 at end of appendix). This decline resulted from a combination of causes including: (1) a drier climatic trend in the 1950's to 1960's and 1970's to 1980's; (2) additional ground-water pumping throughout the Snake Plain aquifer area, with over 400,000 acre-feet in the area adjacent to the North Side Pumping Division; (3) reduced wintertime diversions beginning in the early A&B 572

1970's; (4) reduced irrigation diversions in the Minidoka Irrigation District resulting from water savings practiced by the irrigators; (5) increased pumping in some of the North Side Pumping Division aquifer recharge areas such as Northwest Raft River and Northern Oakley Fan.

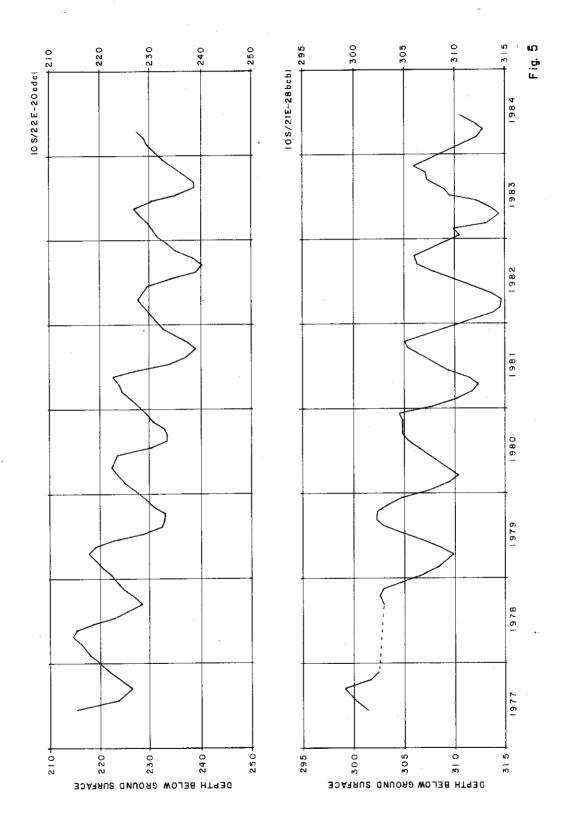
Water level hydrographs (figure 2C, D, and E) show the 1970's to 1982 water level decline has stopped and is showing a slight recovery for 1983 and a stronger recovery for 1984. If this recovery continues a portion of the past decline will be regained.

Additional ground-water pumping has slowed on the Snake Plain because of water rights controversy and because most potential irrigable land is in production. The portion of the decline caused by pumping has slowed and may have stabilized.

Comparison of four water level hydrographs (figure 2C, D, and E) spaced along the Snake Plain show that about 6 to 9 feet of the decline between the 1970's and 1982 was probably caused by climate and other causes such as reduced diversion and any additional decline may be caused by pumping. The total decline may be slightly over 9 feet in the project area. In the southwest part of the project area, hydrographs (figure 5) show the decline slowing and may have stabilized or is showing a very slight recovery. The reason the water levels in this area are behaving differently than elsewhere on the project may be due to ground-water pumping in Northern Oakley Fan.

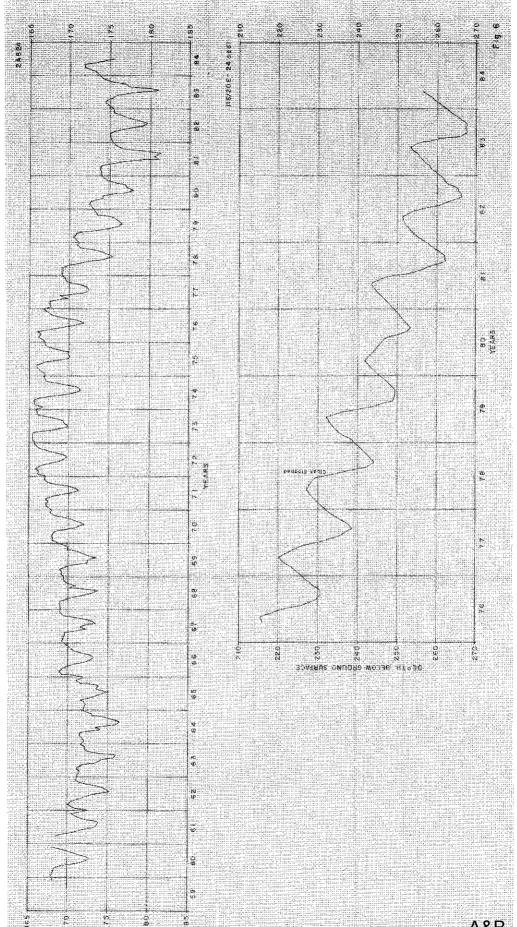
The total net decline in the project area from all causes is about 5 feet from the 1963 low to the 1982 low.

The 1970's to 1982 water level decline period resulted in dropping water levels that caused pump and water yield problems throughout the North Side Pumping Division area. This caused renewed concern about ground-water mining among area pumpers, and stimulated demands for ground-water pumping



restrictions. Reclamation was again requested to study the problem. Review of the previous decline data found the conclusions sound. The current decline problem like the previous one was mostly related to a drier climate trend, but also aggravated by changes in irrigation practices, such as reduced irrigation diversions, throughout the Snake Plain aguifer area. It was concluded that the water level decline was not permanent and, because of a wetter climate trend beginning in about 1983, appears to have begun a recovery. Fear of ground-water mining was also unfounded as shown by comparing water level behavior in the North Side Pumping Division area with that from a local area where ground-water mining is occurring. See figure 6 for comparison of a North Side Pumping Division water level hydrograph (well 2A824) and a ground-water mining hydrograph (well 11S/20E-24dddl). Both hydrographs show yearly fluctuations dropping in spring or early fall and rising in fall or early spring, reflecting influences from major short-term recharge and discharge. The main difference between the hydrographs is shown on the long-term trend. The North Side hydrograph shows decline and recovery, whereas the ground-water mining hydrograph shows a continuing decline of about 40 feet in 9 years with no evidence of stabilizing or recovering. The North Side Pumping Division water levels will continue to decline and recover mostly influenced by climatic changes.

Ground-water mining is caused when discharge exceeds recharge and the decline trend will continue until the resource is exhausted or discharge is reduced to equal recharge. Discharge by pumping from the Snake Plain aquifer presently does not begin to approach recharge. Present net pumping is estimated at about 1.4 million acre-feet per year and recharge is estimated at about 8 million acre-feet per year. Thus, the aquifer can accommodate a substantial increase in ground-water pumping without showing alarming declines.



Summary and Recommendations

Since construction of the pumping division in the 1950's, well construction methods have changed, especially construction specifications written by Reclamation planners. The original 177 project production wells were drilled by drilling contractors using cable drills, and were completed using the usual completion methods at that time. Drilling was continued below the water table until the drill cuttings were "lost," which was apparently an indication of good yield. Construction completion usually consisted of installing surface casing with the balance of the well left "open hole." When caving conditions were encountered during the drilling, a casing liner was installed, generally just through the caving interval. The liner would be perforated when the caving interval was located within the "good" aquifer section of the well. After the well was completed, a pump test was run to determine the yield. If the yield was insufficient, the well would be deepened in hopes of encountering additional water.

These methods were workable, but generally did not allow for much lowering of the pump if the water level declined. The project was begun about the water level peak period and was completed during a water level decline period. More than one-half of the wells had less than 100 feet of saturated well bore; therefore, as the water levels declined, drawdown increased, the thickness of the saturated well bore thinned, and yield decreased. Deepening of many of the wells was undertaken before the project was completed. About one-half of the wells have been deepened to date (1984) and about one-half of the wells still have less than 100 feet of exposed aquifer.

Under the Recommended Plan, 17 wells (13 new and 4 existing) pumping an average of 14,700 acre-feet annually would provide a full irrigation service to 5,680 acres on eight of the Extension areas (5, 6, 7, 10, 13, 14, 15,

and 17). In addition to providing ground water to the eight Extension areas, 620 acre-feet (annual basis) of ground water will be pumped to 240 acres of square-up land scattered throughout Unit B, and 300 acre-feet (based on limited drainage data) of ground water will be pumped annually to supplement drainage flows to Extension Areas 11 and 12. The ground water pumped to the 240 acres of square-up land (620 acre-feet) and to Area 12 (100 acre-feet) will be provided from existing project wells. A new well will be constructed to provide the supplemental ground-water requirements to Area 11 (estimated at 200 acre-feet per year). This brings the total number of wells under the Recommended Plan to 18.

The plan also provides a replacement water supply for 810 acres of North Side Pumping Division land now irrigated from four project wells located near Extension Area 4 which have inadequate ground-water yields. The replacement water supply for these existing irrigated lands would be provided from the Snake River, and the four existing low yielding wells would be abandoned. This would reduce ground-water pumping by 2,100 acre-feet annually. Thus, the total net annual ground-water pumping requirements for the project would be 13,520 acre-feet annually (14,700+620+300-2,100=13,520).

Table 2 is a list of proposed irrigation supply wells and construction specifications for the Extension lands. The depth to water and specific capacity were taken from the previously discussed maps; yearly fluctuations were estimated from well hydrographs and water level data from wells located near each proposed site. An estimated drawdown for each proposed well was based upon specific capacity and design yield. Pump lift is the sum of depth to water, yearly fluctuation, and drawdown and ranges from about 140 feet in tract 13 to about 365 feet in tract 5. The pump suction was placed about 10 feet plus length of the pump below the pump lift or pumping water level.

An additional 50 feet of pump chamber below pump suction was included to allow for pump lowering in case of further water level decline. Thus, the pump chamber includes depth to water, yearly fluctuation, drawdown, 10 feet of pump submergence, length of pump, and 50 feet additional hole. The estimated total depth includes the pump chamber plus additional hole below pump chamber depending upon required yield and well location. Wells with small yield of about 1,000 gallons per minute or slightly more may get by with a total depth near the pump chamber depth. Wells with large yield up to several thousand gallons per minute may require additional hole below the pump chamber.

Maximum total depth may be to 250 feet or more of saturated basalt below the water table. Well diameter will depend upon water volume required at each site. Generally, 20-inch wells are recommended for 2,000 to 3,000 gallons per minute volumes and 24-inch for 4,000 to 5,000 gallons per minute volumes.

During the North Side Pumping Division well drilling in the 1950's, five wells were drilled and not used. These wells are located in tracts 15, 12, 10, and 6 of the Extension lands.

These wells include:

Tract 15-15A724.--A 16-inch diameter, 285-foot-deep well that should yield the required volume as it is constructed.

Tract 12-33A824.--A 20-inch-diameter, 235-foot-deep well that has been sold to the city of Rupert and is not available for use.

Table 2.--Proposed Irrigation Supply Wells and Construction Specifications

		Depth			Yie	ld		200 5020	Estimate	d	-	Well
Tract Number	Well Number	to Water	Yearly Fluctuation	Specific Capacity	Cubic Feet per Second	Gallons per Minute	Pump Length	Drawdown	Pump Chamber	Lift	Total Depth	Casing Diamete
		feet	feet	gal/min ¹ /			feet	feet	feet	feet	feet	inches
5	1	310	10	5,000	8	4,000	10	5	395	325	560	24
3	2	320	10	5,000	8	4.000	10	5	405	335	570	24
3	3	290	20	1.000	12	5,500	10	10	340	320	540	28
	ă	340	15	1.000	9	4,000	10	10	435	365	590	24
	5	300	15	1,000	5	2,000	5	- 5	390	320	400	20
	6	300	10	500	4	1,800	5	5	380	315	400	20
	7	300	15	500	5	2,000	5	- 5	385	320	400	20
	Ŕ	310	10	500	3	1,500	5	5	390	325	400	18
6	ĭ	300	10	1,500	7	3,000	5	5	380	315	500	24
U	ž	300	10	1,500	6	2,500	5	5	380	315	500	24
7		320	15	500	- 5	2,500	5	10	410	345	500	24
10	1	240	10	200	5	2,000	5	25	340	275	500	20
10	2	230	10	3,000	7	3,500	5	5	310	245	500	24
11	-	150	10	1,000	2	900	5	5	230	165	230	16
13		120	10	1,000	7	3,500	5	10	205	140	370	24
14		150	10	1,000	2	1,000	5	5	230	165	250	16
15		220	10	2,000	2	900	5	5	300	235	325	16
17		150	15	500	7	3,000	5	10	240	175	400	22

Pump chamber includes:

depth to water

yearly fluctuation of water table

drawdown

length of pump

10-foot submergence below lift

50-foot additional hole for possible future pump lowering

Total depth of well includes:

pump chamber additional hole depending upon desired yield

1/ Gallons per minute

Tract 10-25A822.--An 8-inch-diameter, 379-foot-deep well that should have a specific capacity of 100 to 500 gpm/ft which is sufficient, but the well is not capable of yielding 2,000+ gpm because of the small well diameter.

Tract 6-8A921.--A 24-inch-diameter, 383-foot-deep well, and 8B921 an 18-inch-diameter, 376-foot-deep well that should have a specific capacity of about 500 gpm/ft and; therefore, after deepening to about 500 feet, should be capable of yielding the required volume for this tract.

Some or all of these wells may need renovation to bring them up to current Reclamation standards. Renovation of the wells will depend upon the cost being enough less than the cost of a new well to justify the necessary work to bring them up to standard.

Generally, deepening is feasible but reaming to a larger diameter is not. According to Mr. Virgil Temple, formerly A&B Irrigation District Manager and

now with Reclamation, Water and Land Division, reaming costs about the same per foot as drilling a new well. The high cost is due to problems encountered and extra strain placed upon the drilling equipment.

Well construction should consist of drilling a hole of adequate diameter to the minimum total depth. The total depth can vary somewhat depending upon where the drill site is selected in each tract. The total depth is determined by selecting a depth where the pump can be placed allowing the pumped water level to remain at least 5 feet above the pump bowls after subtracting out drawdown from pumping and natural fluctuations of the water table. Below the pump intake, a pump chamber is drilled about 50 feet into the aquifer. The pump chamber is essentially that portion of the well where the pump is placed and must be deep enough to allow room to lower the pump in case of persistent water level declines. A well in Southern Tract 10 is in an area where the Specific Capacity of the aquifer is about 200 gpm/ft or less and therefore must have an even longer intake area. The deeper penetration of the aquifer utilizes a greater intake area for increased yield by placing the pump suction deeper to permit more drawdown.

The portion of the well deeper than 50 feet below the pump intake may be reduced in diameter. The reduction should decrease drilling costs and will not materially reduce the intake potential. The Johnson Well Screen Company has found that doubling the well diameter only increases yield by about 11 percent.

Casing must be placed in the upper portions of the well to seal out caving zones in the sediment and prevent aquifer pollution from surface waters. The balance of the well can be left open hole; however, for maximum pump protection, casing should be installed throughout the pump chamber.

The local drawdown radius of influence is rarely detectable in the North Side Pumping Division wells even where two wells are near each other.

Therefore, it is not expected to be a problem with the proposed extension wells.

An additional water table decline of 1 or 2 feet may occur from pumping the new wells. Since the overall decline fluctuates with recharge, the actual decline may not be detectable once the wells are established.

Theoretically, the additional pumping for the Extension lands will cause a decrease in the flow at Thousand Springs. Historically, the decrease in flow at the springs has been less than the net withdrawal. Due to the very small increment of additional pumping relative to the existing pumping and because of lag time and attenuation it is unlikely any change in the flow at Thousand Springs can be identified as the effect of the additional pumping.

CHAPTER 3--WATER RIGHTS

Any new water rights in the upper Snake River are in question because of the 1982 Idaho Supreme Court ruling. Therefore, a perspective of the so-called Swan Falls controversy is necessary to provide the setting for the water right situation for the North Side Pumping Division Extension.

Idaho Power Company's existing Swan Falls Powerplant is located on the Snake River south of Boise and downstream from the project area. Swan Falls Dam and Powerplant were built in the early 1900's. The Idaho Supreme Court ruled in 1982 that the Swan Falls water rights are not subordinate to later upstream depletions which have been made by irrigation. Under the subordination concept, the power rights would have been inferior to subsequent upstream irrigation water uses. The Supreme Court remanded to the District Court the question as to whether the Idaho Power Company has abandoned or eroded through nonuse some of its Swan Falls water rights.

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The lengthy time and costs involved to resolve the Swan Falls issue through litigation may not have necessarily yielded a solution which would have reflected the interests of the participants as well as the public. Therefore, in an attempt to settle the problem, the Idaho Power Company and the State of Idaho identified and mutually agreed upon the following series of judicial, legislative, and administrative actions which should be taken in the public's interest:

- 1. The Snake River minimum streamflow at the Murphy gauge in the State water plan should be adjusted to $5,600~\rm{ft}^3/\rm{s}$ during the nonirrigation season and $3.900~\rm{ft}^3/\rm{s}$ during the irrigation season.
- 2. Each new development should be carefully scrutinized against public interest criteria.
- 3. The State should commence a general adjudication of the entire Snake River basin.
 - 4. The State should establish an effective water marketing system.
- 5. State funding should be provided for hydrologic and economic studies to implement the State water plan in the most cost-effective and environmentally sound means.
- 6. Legislation should be enacted to clarify that proceeds from utility sales of hydropower water rights will benefit ratepayers. In March 1985, the required legislation was enacted by the Idaho State Legislature to modify and add the necessary language to Idaho Code which allows full realization of the negotiated contract between the State of Idaho and the Idaho Power Company.

The existing A&B Irrigation District water rights discussed in the "Water Resources" chapter have not been affected by the Swan Falls issue; however, the potential water supplies for the Extension project could be affected by this agreement. The Idaho Department of Water Resources will again begin

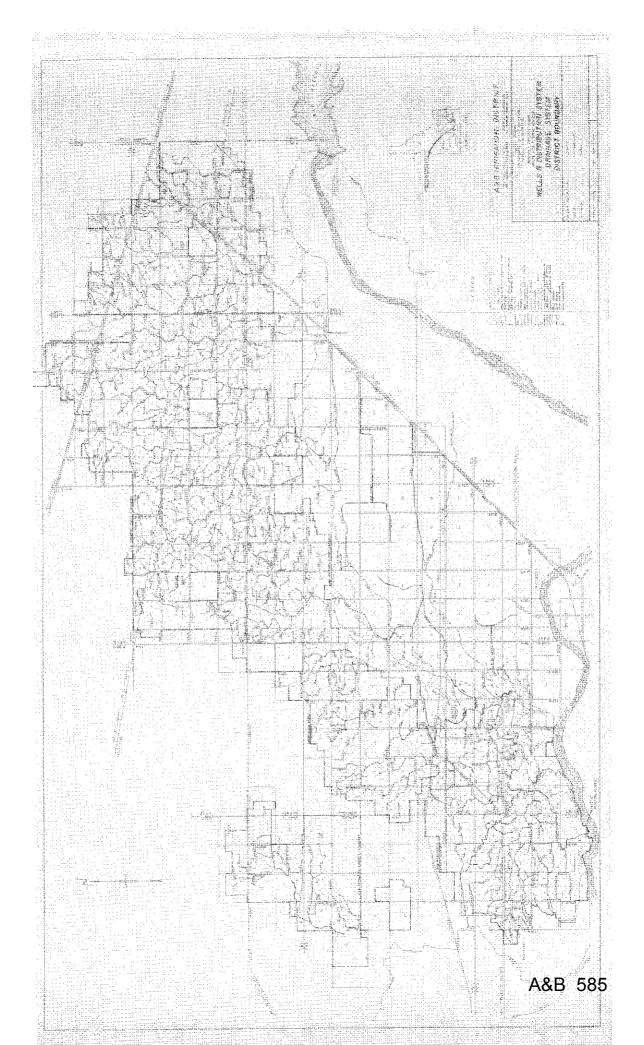
processing water right applications in the not-to-distant future. A ground-water right application (No. 368253) for the new Unit B lands was filed on November 23, 1984, and a surface-water right application (No. 01-7085) for the new Unit A lands was filed on April 22, 1985. The Idaho Department of Water Resources is defining the rules for new applications; Reclamation is working closely with Idaho Department of Water Resources to insure that all proposed water uses are covered by valid State water rights.

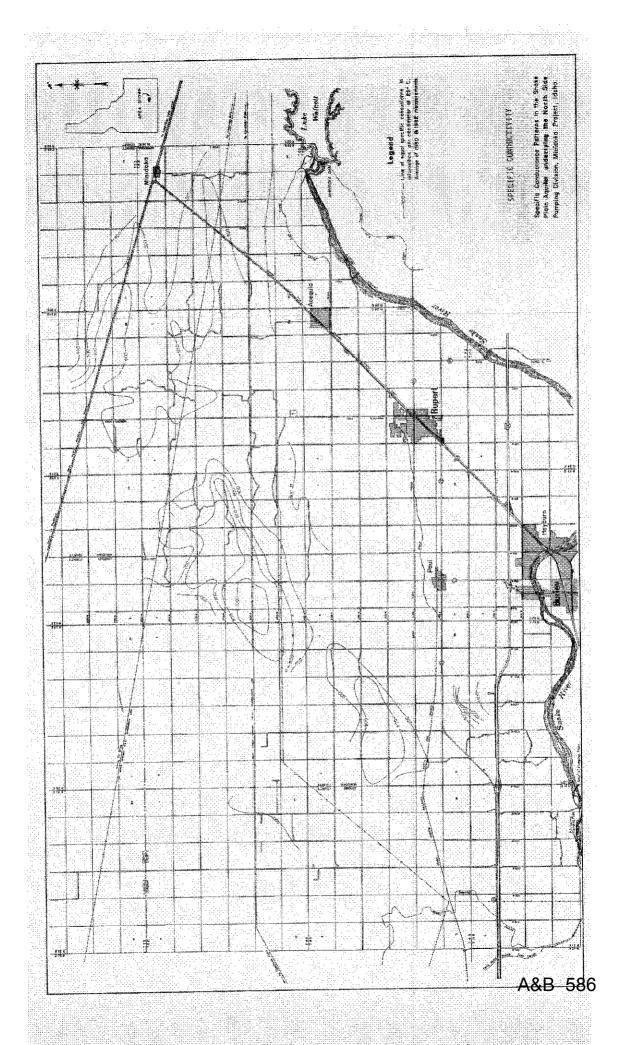
The American Falls and Palisades storage water rights discussed in "Water Resources" chapter were reviewed; about 790 acres of new Unit A lands were not included in the American Falls storage right, and about 1,200 acres of new Unit A lands were not included in the Palisades storage right. However, included in each storage right are Unit B lands served with ground water which do not utilize storage water. To include these new lands in the American Falls and Palisades storage rights, applications to change the place of use were filed with the Idaho Department of Water Resources for each reservoir. These applications transfer the storage water right of part of the Unit B lands discussed above to the new Unit A lands not included in the original filings.

CHAPTER 4--WATER OUALITY

Irrigation water supplies for the North Side Pumping Division are withdrawn from the Snake River and from a series of wells on the project.

Most surface drainage is accomplished by the use of injection wells which pass water directly into the underlying aquifer. Project wells, distribution, and drainage systems are shown on the map on the following page.





Irrigation Water Quality

The quality of water both in the Snake River (table 3) and in ground water pumped from the Snake Plain aquifer (table 4) is good for irrigation purposes. The electrical conductivity of production wells on the North Side Pumping Division was found to range from about 400 micro siemens per centimeter (uS/cm) to slightly above 1,000 uS/cm in a Bureau of Reclamation survey (see Specific Conductivity map). Although yield reductions on sensitive crops such as beans would be expected when irrigated with water having an electrical conductivity greater than 750 uS/cm, beans are generally not grown on areas of the project irrigated with ground water. The low sodium adsorption ratios of surface- and ground-water supplies indicate a low sodium (permeability) hazard. Further, soils in the area have good internal drainage, and no salt problems have occurred after irrigating more than 20 years.

Table 3.--Mean Value and Standard Error of the Mean for Water Quality Parameters from the Snake River near Burley, Idaho (Geological Survey Data)

Parameter	Snake River near Burley	Irrigation Criteria
T GT GINE DET	near barres	Of FOCI TO
Elęçtrical conductivity (uS/cm)	445 + 8	<750
pH ¹ / (standard units)	8.2 ± 0.1	4.5-9.0
Boron $(mg/L)^{\frac{2}{2}}$		<0.75
Calcium (mg/L)	47.0 + 1.0	
Magnesium (mg/L)	15.6 ± 0.3	
Sodium (mg/L)	19.27 ± 0.84	
Sodium adsorption ratio (adjusted)	19.27 ± 30.84 1.24	<3
Chloride (mg/L)	19 + 1	<142
Bicarbonates (mg/L)	190 + 4	
Fecal coliform bacteria (counts		
per 100 mL ^{4/})	11 <u>5</u> /	<1,000

^{1/} Hydrogen-ion concentration

^{2/} Milligram per liter

^{3/} Calculated from mean values

^{4/} Milliliter

 $[\]overline{5}$ / Geometric mean

Table 4.--Water Quality Characteristics of Representative Production Wells on Unit B, North Side Pumping Division--Chemical Data Were Collected in July 1977, Bacterial Data June 1984

Parameter	32A725	15D823	35D823	118922	Irrigation Criteria
Electrical conductivity (uS/cm)	440	840 <u>3</u> /	583	857 <u>3</u> /	<750 <u>1</u> /
pH (standard units)	7.9	7.8	7.5	7.8	$4.5-9.0^{2}$
Boron (mg/L)	0.12	0.15	0.08	0.15	<0.75 ² /
Calcium (mg/L)	36.9	64.3	64.3	77.2	
Magnesium (mg/L)	17.3	30.5	19.3	28.7	: (***)
Sodium (mg/L)	27.13	59.77	29.66	59.54	\$
Sodium Adsorption Ratio (adj. SAR)	1.65	$3.60^{3/}$	0.97	2.10	<3 <u>1</u> /
Chloride (mg/L)	34	83	27	57	<142 <u>1</u> /
Carbonates + bicarbonates (mg/L)	161	279	269	353	
Fecal coliform bacteria (counts /100 ml)	<1	<1	<1	<1	<1,0002/

^{1/} From: Ayers, R.S., and R.L. Branson, 1975, Water Quality Guidelines for Interpretation of Water Quality for Agriculture. Mimeograph, University of California Coop Extension Service.
2/ From Environmental Protectin Agency, 1972, Water Quality Criteria, Publication No. EPA-R3-73-033, U.S.

Government Printing Office, Washington, D.C. 3/ Problems for sensitive crops such as beans only (not generally cultivated on lands irrigated with ground water); soil permeability unaffected by SAR value

Drainage Water Quality

Most irrigation return flows and storm runoff are disposed of through injection wells which pass water directly into the Snake Plain aquifer. Injected wastewaters occasionally contain sediments and coliform bacteria in excess of Idaho drinking water standards (Graham, Clapp, and Putkey, 1977) and have been linked to contamination of domestic wells in the area. Pesticides, herbicides, and trace metal concentrations in irrigation wastewaters were found to be within drinking water standards (Graham, Clapp, and Putkey, 1977). Careful management is required to prevent and correct domestic water quality problems.

Irrigation return flows entering disposal wells on the North Side Pumping Division do not consistently comply with Idaho standards for injected waters (table 5). Because of the potential for contamination of the Snake Plain

aquifer, Idaho regulations may require some changes in the way project injection wells are operated.

Table 5.--Range of Values for Chemical and Physical Parameters from Water Entering Representative Drain Wells on the North Side Pumping Division

Parameter	22G825	2480824	Well 13AD1021	48D923	26CD823	Idaho Standard3/
Turbidity (Hach FTU)	2-140	13-260	34-90	2-7	55-300	100
Electrical Conductivity (uS/cm)	487-677	463-635	382-635	697-805	759-853	
pH (standard units)	8.0-8.9	7.4-8.5	7.6-8.6	8.3-8.6	8.0-8.4	**
Suspended Solids (mg/L)	5-560	19-692	93-189	7-20	69-1,473	
Nitrate + Nitrate-N (mg/L)	0.28-0.78	1.14-1.35	0.12-1.50	3.29-3.80	3.24-4.40	10
Total Organic Carbon $(mg/L)^{\frac{1}{L}}$	1.6	12.0		2.2	••	10
Color (Platinum-Colbalt units)2/	<5-5	<5		<5-5		100
Odor (TON at 60 degrees C) ^{2/}	<5-5	<5-5		<5-5	++	30
Fecal Coliform Bacteria (counts/100 ml) ²⁷	80-330	200-3,300		10-400		500
Total Coliform Bacteria (counts/100 ml)—	700-1400	7000		1000-6000		10,000

The injection well problem on the A&B Irrigation District was not addressed in the Extension study because of the short time frame, except that reuse of existing drain flows was incorporated into the proposed plan wherever feasible.

Considerable work has already been done by the State of Idaho (1983) and the Bureau of Reclamation on alternatives to the injection wells. Additional study efforts by Reclamation and A&B Irrigation District to gather necessary data and to develop a feasible solution to the injection well problem will continue concurrently with and beyond the Extension study.

^{1/} Results from single sample, July 1984 2/ Results from 2 samples collected in June and July 1984 3/ Existing drain wells

Impacts of Proposed Plan

The proposed Extension plan would involve a net pumpage of about 13,520 acre-feet from the Snake Plain aquifer and $12,300^{1/2}$ acre-feet from the Snake River. Present use of the Snake Plain aquifer for irrigation includes approximately 210,000 acre-feet per year on the North Side Pumping Division and an estimated total of 600,000 acre-feet per year on the entire lower Snake Plain area between Rupert and Thousand Springs, Idaho. Current average annual irrigation diversions from the upper Snake River system are estimated to be 8.2 million acre-feet per year.

Since all new Extension lands will be irrigated with sprinklers, return flows will be minimized and held in ponds for reuse or loss through evaporation and deep percolation. Although some increases in dissolved salts and the resultant electrical conductivity of the Snake Plain aquifer and Snake River could occur through deep percolation of return flows, this would be a slight impact, if measurable, because return flows from Extension lands would be a minor source of water and salts to both the aquifer and river. Since no return flows from new irrigation will be disposed of in injection wells, Extension lands would not contribute to bacterial and sediment problems in the Snake Plain aquifer.

Potential for reuse of existing irrigation return flows was examined under the Extension study as a partial solution to the injection well problem on the North Side Pumping Division. Reuse of drainwater from H Main Drain, (now entering drain wells 4BD923 and 5AD823, see A&B Irrigation District map) E Main Drain, and the F Main Drain (now entering drain wells 9AD824, 11AD823,

^{1/} The proposed surface-water pumping of 12,300 acre-feet includes 2,500 acre-feet for 810 acres of land presently served with ground water that would be served with Snake River water.

32AD824, and Cap Hawley) were found feasible; however, a new irrigation well would be required to augment the H Main Drain flows for much of the irrigation season, and existing project wells would be required to augment both early and late season drainwater flows in the E and F Main Drains. The records maintained on the drains and injection wells in the A&B Irrigation District indicate there is about 100 acre-feet per year available to the new lands at the terminus of H Main Drain (Area 11) and about 1,200 acre-feet per year available to the new land between E and F Main Drains (Area 12).

The quality of drainwaters proposed for reuse is suitable for irrigation (table 6). Reuse should produce a minor reduction in bacterial and sediment contamination of the Snake Plain aquifer.

Table 6.--Water Quality Characteristics of Drainage Water Proposed for Reuse under the North Side Pumping Division Extension Study--Data Were collected in September 1984

Parameter	"H Main" 4BD923	32AD824	"F Main" Cap Hawley	9AD8244/	Irrigation Criteria
Electrical conductivity (uS/cm)	785	756	646	632	<750 <u>1</u> /
pH (standard units)	8.4	8.3	8.0	8.0	4.5-9.02/
Boron (mg/L)	0.33	0.55	0.58	- 2	<0.75 <u>2</u> /
Calcium (mg/L)	60.5	61.5	55.9		
Magnesium (mg/L)	31.4	27.5	23.5	:	••
Sodium (mg/L)	64.14	57.48	45.75		••
Sodium Adsorption Ratio (adjusted)	3.58	3,30	2.79		<3 <u>1</u> .
Chloride (mg/L)	79	80	62		<142 <u>1</u>
Carbonates + bicarbonates (mg/L)	265	248	227		
Fecal coliform bacteria (counts/100 ml)	30	400	600		4,0002

^{1/} From: Ayers, R.S. and R.L. Branson, 1975, Water Quality Guidelines for Interpretation of Water Quality

4/ Data collected in September 1983.

for Agriculture. Mimeograph, University of California Coop Extension Service.

2/ From: Environmental Protection Agency, 1972, Water Quality Criteria, Publication No. EPA-R3-73-033, U.S. Government Printing Office, Washington, D.C.

^{3/} Problems for sensitive crops, such as beans only (not generally cultivated on lands irrigated with ground water); soil permeability unaffected by Sodium Adsorption Ratio.

The Environmental Protection Agency has received a petition requesting designation of the Snake Plain aquifer as a sole source of drinking water. If the Snake Plain aquifer is designated a sole source of drinking water, Environmental Protection Agency would be required to review and approve projects which might contaminate the aquifer. It seems unlikely that sole source review would result in any changes in the proposed Extension plan because (1) no drainage water from Extension lands would be disposed of in existing drain wells, (2) no new drain well would be drilled to dispose of drainwater, and (3) some water now entering injection wells would be applied to new lands.

Future without Proposed Plan

The quality of water pumped from the Snake River and the Snake Plain aquifer to serve North Side Pumping Division lands is suitable for presently irrigated crops and is expected to remain suitable in the future. No major changes in the concentration of inorganic salts are expected in water supplied from either the Snake River or the Snake Plain aquifer since little additional irrigation development is projected in areas upgradient of the North Side Pumping Division.

Most surface drainage on North Side Pumping Division lands is accomplished through the use of injection wells which pass water directly into the Snake Plain aquifer. Irrigation return flows generally contain fecal coliform bacteria in excess of Idaho drinking water standards and have been linked to contamination of domestic wells in the area. Bacterial contamination of domestic wells in the Snake Plain aquifer is expected to decline in the future as problem disposal wells are identified, pollution sources cleaned up, and alternatives to present wastewater injection practices implemented as required by Idaho regulations governing use of dispass wells.

If the Snake Plain aquifer were designated as a sole source of drinking water, this designation is not expected to impact operation of existing irrigation and drainage facilities on the North Side Pumping Division because sole source review is not retroactive.

CHAPTER 5--IRRIGATION WATER REQUIREMENTS

Summary of Results

Average monthly diversion requirements and peak delivery requirements were estimated for the period 1965 to 1982 using representative climatological data. It was assumed all newly irrigated lands would be served by sprinkler systems having an irrigation efficiency of 70 percent. The delivery system serving lands in Unit A differs from the system serving lands in Unit B; therefore, separate water requirements were derived for each unit.

Monthly Diversion Requirements

The average monthly and the average annual diversion requirements are presented in tables 7 and 8, respectively. These diversion requirements are adopted for use in project planning studies.

Table 7.--Average Monthly Diversion Requirements

		Diversion Req	
Month	Distribution	Unit A	Unit B
	percent	inches pe	r acre
April	1.7	0.65	0.54
May	8.7	3.25	2.71
June	25.9	9.67	8.04
July	36.3	13.55	11.27
August	17.7	6.62	5.51
September	7.5	2.82	2.35
October	2.2	0.81	0.67
Total	100.0	37.37	31.09
		(3.11 feet)	(2.59 feet)

Table 8.--Annual Diversion Requirements

Area	Annual Diversion Requirement
acres	acre-feet
3,950	12,300
	1/
	$1,500\frac{1}{}$
5,680	14,700
10,210	28,500
	3,950 580 5,680

^{1/} Drainwater would need to be augmented with ground-water pumping. For Area 12, existing project wells would be required to augment both early and late season drainwater flows. A new well would be required to augment H Main Drain flows for much of the irrigation season to serve lands in Area 11.

Design Peak Delivery Rate

A peak farm delivery rate of 0.434 inch per day was estimated during the course of this study. This rate is well within acceptable limits; however, the ABID views the development of the Extension lands as a completion of the original project. In a letter to the Bureau of Reclamation dated May 24, 1984, the district states that they cannot support a peak net farm delivery in excess of 0.357 inch per day, which is the rate at which the current project is designed and operated. Therefore, a peak farm delivery of 0.357 inch per day was adopted for use in this study. This rate represents the capacity at the turnouts. Additional capacity is required for conveyance losses in the laterals and canals.

General

The irrigation water requirements are discussed in two sections. In the first section, monthly diversion requirements are computed based on A&B 594

^{2/} Does not include 240 acres of square-up lands scattered throughout Unit B. The annual diversion requirements for this land is 620 acre-feet and will be provided from existing project wells in Unit B.

monthly consumptive use (evapotranspiration) estimates. In the second section, a peak delivery requirement is estimated and used to develop pump, canal, and lateral sizing criteria.

All consumptive use estimates (monthly and peak) are accomplished using the Jensen-Haise procedure assuming a mean project elevation of 4250 feet.

The Jensen-Haise equation for estimating reference evapotranspiration is:

$$E_{tr} = C_t (T - T_x) R_s$$

where: $E_{\rm tr}$ is reference ET based on alfalfa with 30 to 50 centimeters of top growth, inches per day.

T is mean daily temperature, °F; T is the temperature axis intercept, °F; $T_x^X = 27.5 - 0.25 (e_2 - e_1) - (elevation/1000).$

R is global solar radiation, calories per square centimeter per sday (langleys/day), (multiplied by 0.000673 to obtain inches per day).

 C_{T} is an empirical coefficient;

$$C_t = 1/(C_1 + C_2 C_H)$$
 $C_1 = 68 - (3.6 \times elevation/1000)$
 $C_2 = 13$
 $C_H = 50/(e_2 - e_1)$

e, and e, are saturation vapor pressure (e, in millibar) of water at the long-term (normal) mean maximum and mean minimum temperatures, respectively, for the warmest month (usually July) of the year for the study area.

The term "reference ET" has replaced "potential ET;" however, the computer output labeling has not been updated. Therefore, in this study, the two terms should be considered synonymous.

Section I -- Monthly Diversion Requirement

Diversion requirements were derived by first determining the average monthly consumptive use of the crops projected to be grown. The basis of the ABB 595

Jensen-Haise procedure is that consumptive use is a function of temperature and solar radiation given an adequate water supply. The effective precipitation is then subtracted from the estimated consumptive use to obtain the water needed to be supplied by irrigation. A farm irrigation efficiency is then applied to the crop irrigation requirement to obtain the farm delivery requirement. Losses associated with this efficiency are assumed to occur because of wind drift, application evaporation, runoff, and deep percolation. Expected seepage losses from conveyance systems along with operational spills are added to the farm delivery requirement to obtain the diversion requirement.

Consumptive Use.—The computer program XCIR was employed to aid in the computation of the monthly crop irrigation requirements. The program first computes reference evapotranspiration ($E_{\rm tr}$) from monthly values of solar radiation and temperature and then calculates crop evapotranspiration (ET) by multiplying the $E_{\rm tr}$ by an appropriate crop coefficient. The program also computes that portion of precipitation considered available to satisfy crop evapotranspiration requirements. Effective precipitation is a function of the total precipitation, rainfall intensity, infiltration rate of the soil, water holding capacity of the soil, and the evapotranspiration rate. The crop irrigation requirement is then computed as crop ET less the effective precipitation and any nongrowing season soil moisture carryover. Any excess soil moisture is considered runoff or deep percolation.

Data Input to XCIR.--Input data to the XCIR program includes average monthly temperature, monthly precipitation, average monthly solar radiation, maximum water holding capacity, and the projected crop distribution for the project area. Other input included the long-term average maximum and minimum temperatures in July, the growing seasons and cover dates for each crop, and crop coefficients.

Temperature and Precipitation.—Daily temperature and precipitation data have been collected continuously from May through September on the Minidoka North Side Project as part of an Irrigation Management Service (IMS) program. To utilize this data, a substantial number of temperature and precipitation values would have to be estimated for April and October because the growing season for the majority of the crops in this study begins in April, with approximately 50 percent of the crops growing into the month of October. Further, this data does not lend itself to utilizing the effective precipitation and carryover soil moisture routine in XCIR because this routine requires average monthly climatological data for the entire year. For these reasons an alternate weather station was selected to represent the Minidoka North Side Project.

The Twin Falls Weather Service Office (WSO) has collected daily temperature and precipitation data on a long-term basis. This station is located at the Idaho Water and Energy Resources Research Institute (Snake River Conservation Research Station, U.S. Department of Agriculture-Agricultural Research Service), Kimberly, Idaho, approximately 36 miles southwest of the project area. Although other stations were closer to the project area, this weather station was selected primarily because of its surrounding agricultural environment. Further, it is the nearest station which collects daily solar radiation throughout the year.

The Twin Falls average monthly temperature and precipitation data for a 17-year period (1965-72 and 1974-82) was compared to the IMS average monthly climatological data for the May through September period. As shown in table 9, the average monthly temperatures at Twin Falls are approximately 1 °F higher than the average monthly IMS temperatures. The average monthly

precipitation data compares quite favorably with the only exception occurring in September. In this case, the Twin Falls precipitation is slightly higher.

Table 9.--Comparison of Average Monthly Temperature and Precipitation, 1965-72 and 1974-82

Station	Climatic Variable	May	June	July	August	September 1/
IMS	Temperature (°F) Precipitation	52.9	61.1	68.0	66.3	57.3
	(inches)	1.07	0.79	0.31	0.57	0.61
Twin Falls	Temperature (°F)	53.7	62.1	69.1	67.3	58.2
(WSO)	Precipitation (inches)	1.03	0.78	0.33	0.53	0.83

^{1/} Represents the period 1965-72 and 1978-82

The consistent difference in temperature (1 °F) may be explained by the following reasons: first, the differences in the calibration of the temperature recording devices and second, the elevation of the Twin Falls station is 3960 feet which is 250 feet lower than the IMS station.

Three additional stations (adjacent to the project) were selected for annual temperature comparisons to assure the Twin Falls station was representative of the project area. They are Burley Federal Aviation Administration (FAA) at elevation 4157 feet; Minidoka Dam at elevation 4210 feet; and Paul at elevation 4210 feet. The average annual temperatures are compared for each station from 1965 through 1982 exclusive of 1975-76 period (Minidoka Dam has missing data in this period) in table 10.

Table 10.--Comparison of Average Annual Temperatures, 1965-74 and 1977-82

	Twin Falls	Burley FAA	Minidoka Dam	Pau 1
Temperature (°F)	47.7	47.8	48.1	47.1

There are no significant temperature differences among the four stations; therefore, it was assumed the Twin Falls station was representative of the project area and its data was used in the consumptive use computations. The period selected for computation was restricted to 1965 through 1982 due to the availability of the solar radiation data at the Twin Falls station. The monthly temperature data and precipitation data are summarized in tables 11 and 12, respectively.

Table 11.--Monthly Average Temperature in Degrees (Estimate), Temperature Data, Twin Falls WSO Elevation = 3960 Feet

	TEMPERAT	URE DA	TA,TWIN	FALLS	450 ELEV	ATION=	3960 FT	•				
YEAR	JAN.	FEB.	MARCH	APRIL	MAY	JUNE	JUL Y	AUG.	SEPT.	OCT.	NOV.	DEC.
1965	31.3	34.0	34.9	48.9	51.2	60.5	58.1	65.9	53.4	51.9	42.5	29.5
1966	29.4	30.5	38.8	46.1	58.5	62.1	70.2	67.8	62.7	46.9	40.5	29.3
1967	33.0	36.2	39.8	41.3	54.1	60.2	72.3	71.9	63.4	47.5	39.5	24.8
196B	27.5	36.3	42.0	43.3	54.1	62.8	70.7	62.6	57.1	47.4	37.3	28.6
1969	31.2	30.0	36.1	48.4	58.9	61.8	69.4	69.2	61.5	43.3	38.5	32.2
1970	32.7	38.0	38.3	40.6	53.8	62.7	69.6	69.9	53.5	44.8	39.1	28.2
1971	30.8	33.1	36.4	44.1	54.9	60.8	69.4	71.3	53.5	44.9	37.Q	25.7
1972	26.0	31.4	42.7	44.6	54.8	63.6	67.3	68.5	54.6	47.8	36.1	23.5
1973	26.0	32.6	38.2	44.7	56.3	62.4	68.5	67.6	56.9	49.1	37.5	33.3
1974	24.9	33.6	38.2	45.3	53.4	66.2	68.9	65.8	59.5	48.5	38.5	30.2
1975	24.8	30.3	37.6	39.4	50.2	60.6	72.4	64.0	59.2	47.9	35.6	33.8
	28.B	29.6	33.4	44.7	55.9	59.7	69.3	63.4	59.6	47.4	39.3	29.9
1976 1977	23.0	33.8	36.8	49.8	50.1	66.6	68.1	67.9	57.4	49.5	37.2	34.9
	33.6	34.3	43.8	45.9	51.4	61.0	67.8	64.2	55.9	49.7	34.3	25.1
1978	16.1	31.7	40.1	45.0	55.5	62.7	68.6	67.2	62.9	51.6	32.8	32.0
1979	26.1	36.6	38.0	48.9	53.1	60.4	68.9	64.7	59.0	48.3	37.7	33.1
1980		33.9	41.2	47.9	52.9	61.7	67.8	70.0	60.1	45.4	40.4	33.5
1981	31.8			42.0	52.0	61.4	66.6	69.0	58.2	46.1	34.2	29.5
1982	23.3	26.2	39.2	→2. 0	52.0	41.4		-0.0				
MEANS	27.8	32.9	38.6	45.0	53.9	62.1	69.1	67.3	58.3	47.7	37.7	29.8

Table 12.--Monthly Precipitation, Twin Falls WSO

PRECIP	ITATION	IN INCH	EŞ			VALUES B	ASED ON	ESTIMATE	TEMP A	ND OR PR	ECIP		
YEAR	JAN	FEB	MAR	APR	MAY	JUN	JUL	AUG	SEP	OCT	NOV	DEC	TOTA
1965	.79	. 21	. 18	2.08	1.14	.51	. 38	.87	. 60	0.00	.74	. 79	8.29:
1966	. 36	. 22	. 85	. 26	. 33	. 10	. 33	.02	. 16	. 28	1.15	. 56	4.62:
1967	. 80	. 10	1.20	1.90	. 58	2.38	. 29	.01	. 28	. 94	. 36	. 17	9.01:
1968	.76	1.68	1.00	.44	. 93	. 93	0.00	3.23	. 29	31	1.62	1.97	13.16:
1969	. 5 1	. 76	.08	. 27	. 12	1.01	. 16	0.00	. 85	. 43	.68	1.23	6.10:
1970	3.38	. 19	. 57	.75	1.10	1.62	1.09	. 05	1.03	. 58	2.32	1.09	13.77:
1971	2.09	. 31	. 95	2.04	2.00	1.40	.01	. 29	. 75	. 71	1.90	1.05	13.50:
1972	2.81	1.18	1.14	. 35	. 32	. 90	.01	. 32	.92	1.32	.89	1.36	11.52:
1973	. 97	. 38	1.84	1.00	. 60	1.07	. 50	. 07	. 89	. 26	2.10	1.78	11.46:
1974	. 75	. 77	1.91	.70	. 15	. 25	. 27	. 08	0.00	1.19	. 37	1.31	7.75:
1975	. B4	1.71	2.18	1.44	2.79	. 37	. 45	. 06	. 02	2.59	. 86	. 49	13.80:
1976	1.52	1.29	.43	. 87	.41	. 81	. 26	2.02	.79	.72	. 10	0.00	9.22:
1977	. 34	. 22	. 18	. 14	1.86	.72	1.27	. 36	.78	.04	1.77	2.03	9.71:
1978	1.32	. 83	1.72	1.40	1.19	. 14	. 12	. 22	2.41	.01	. 52	. 30	10.18:
1979	2.28	. 49	1.13	. 32	.73	. 46	. 11	.78	02	.83	.82	. 32	8.29
1980	2.71	. 88	. 34	. 88	2.74	. 99	. 18	. 11	1.73	. 34	. 77	1.04	12.71
1981	. 5 1	. 44	. 45	1.92	. 76	. 28	.01	0.00	. 35	1.21	1.72	2.95	10.60
1982	.77	1.08	1.38	. 94	. 39	. 52	. 70	. 43	1.48	1.60	. 76	1.25	11.30
AVE	1.31	.71	. 97	. 98	1.01	. 80	. 34	.50	.74	.74	1.08	1.09	10.28

Additional temperature input required for XCIR program is the average daily maximum and minimum temperatures for the warmest month. These average daily temperatures are utilized in the program to compute the temperature coefficient (C_t) and the temperature intercept (T_x). The warmest month for this area generally occurs in July, and the average daily maximum and minimum July temperatures for the 18-year period (1965-82) are 85.0 °F and 53.2 °F, respectively.

Solar Radiation.--Daily solar radiation data was also collected on the Minidoka North Side Project as part of the IMS program. For the reasons discussed in the temperature and precipitation section above, an alternate solar radiation station was selected to represent the project area.

The Twin Falls weather station average monthly solar radiation data is available from 1965 through 1982. As shown in table 13, the Twin Falls average monthly solar radiation data is very close to the average monthly IMS solar radiation (the period selected for comparison represents the available IMS data). Therefore, it was assumed the Twin Falls data was

representative of the project area and was adopted for use in the consumptive use computations.

Table 13.--Comparison of Solar Radiation Data

	Monthly Average1965-72 and 1974-82								
Station	May	June	July	August	September∸				
<u></u>			langleys p	er day					
Minidoka IMS	548	616	632	549	434				
Twin Falls WSO	566	620	636	545	428				

^{1/} September represents 1965-72 and 1978-82

The average monthly solar radiation from 1965 through 1982 is presented in table 14.

Table 14.--Monthly Average Solar Radiation in Langleys, Solar Radiation, Twin Falls WSO Elevation = 3960 Feet

	SOLAR	RADIATIO	N, TWIN	FALLS	WSO ELE	-MOITAV	3960 F	Γ.				
YEAR	. ۸۵ ل	FEB.	MARCH	APRIL	MAY	JUNE	JULY	AUG.	SEPT.	OCT.	NOV.	DEC.
1965	143	290.	436.	464.	536.	607.	628.	537.	449.	352.	189.	149.
1966	159		384.	520.	620.	634.	663.	571.	431.	323.	191.	152.
1967	135		341.	404	596.	562.	618.	579.	426.	315.	178.	132.
1968	175		373.	506.	590.	622.	675.	467.	454.	322.	185.	135.
1969	157		407	527.	625.	568.	657.	574.	435.	297.	213.	116.
1970	136		336	493.	584.	596.	618.	557.	438.	310.	184.	138.
1971	159	. –	356.	418.	557.	620.	661.	540.	446.	297.	168.	153.
1972	182		369.	477.	571.	627.	668.	540.	406.	277.	158.	160.
1973	163		357.	499.	631.	649.	635.	560.	445.	314.	147.	106.
1974	153		344.	515.		705.	658.	603.	518.	29 9 .	188.	125.
1975	194	-	316.	392.	540.	621.	611.	571.	492.	284.	197.	145.
1975	166		391.	465.	611.	667.	634.	522.	441.	334.	227.	175.
1977	149	-	334.	521.	46B.	603.	616.	487.	414.	310.	167.	114.
1978	138		338.	391.	540.	642.	621.	561.	399.	334.	165.	145.
1979	175		352.	463.	573.	639.	618.	469.	453.	279.	200.	134
1980	150		319.	434.	478.	557.	573.	519.	372.	306.	184.	138
1981	159		356.	466.	505.	646.	697.	606.	471.	254.	184.	138
1982	173		303.	479.	595.	628.	603.	56 6 .	390.	292.	185.	129
MEANS	159	248.	356.	469.	5 70.	622.	636.	546.	438.	306.	184.	138

Crop Distribution and Growing Season.--The project crop distribution of the Minidoka North Side Extension is shown in table 15. The pattern is primarily based on historical cropping patterns of the A&B Irrigation District

and the Minidoka Irrigation District. The current historical pattern of the A&B Irrigation District would have been used except the Extension lands are expected to have more forages and less grains than the existing A&B Irrigation District cropping pattern.

Table 15.--Cropping Pattern

Crop	Percentage		
Barley (spring)	15		
Wheat (2/3 spring, 1/3 winter)	24		
Dry beans	8		
Sugar beets	26		
Alfalfa hay	_27		
Total	100		

Key planting, cover, and harvesting dates for the most prevalent crops grown near Kimberly, Idaho, are published by the Bureau of Reclamation in the "Technical Guideline for Estimating Agricultural Crop Water Requirements" dated September 1983. In 1970, the Soil Conservation Service (SCS) published an "Irrigation Guide for Southern and Southeastern Idaho" containing planting and harvesting dates for 17 crops grown in the Rupert, Idaho, area. These planting and harvest dates in the SCS guide for the above five crops compared favorably with the respective Reclamation technical guideline dates. The SCS irrigation guide lacks cover data information; therefore, the Reclamation irrigation guideline data was selected for the consumptive use computations. The appropriate growth stage data used in the XCIR program is illustrated in table 16.

Table 16. -- Growth Stage Data

		M	lonth/Day	ý	Time (Days)		
Crop		1	Full		Planting to	Full Cover	
		Planting	Cover	Harvest	Full Cover	to Harvest	
Spring grain	s						
(barley, wheat)		4/04	6/20	8/10	80	5 5	
Winter wheat		$\binom{2/15}{(10/10)^{1/2}}$	CIOC	8/10	110	60	
Dry beans Sugar beets		5/22	7/15	8/30	55	45	
		4/15	7/10	10/15	85	95	
Alfalfa hay cuttings	1st	4/01	5/01	6/15		76	
cuccings	2nd	6/15	-,	8/01		46	
	3rd	8/01		9/15		46	
	4th	9/15		10/30		46	

^{1/} Effective planting data used for computations; effective germination is spring green-up, actual dates in parentheses.

Soil Moisture. -- A research program on "Use of Water on Federal Irrigation Projects" was conducted by Reclamation on the Minidoka North Side Pumping Division from 1964 through 1968. Soil moisture data was collected as part of this program for selected crops grown in the project area. This data is inclusive of the five crops in the proposed crop distribution and is summarized for each crop and the weighted crop distribution in table 17. The available soil moisture is defined as the amount of moisture available to the plant between field capacity (one-third atmosphere) and permanent wilting point (15 atmospheres). The usable soil moisture is the amount of soil moisture depletion at which optimum crop yields can be maintained.

^{2/} Effective planting data for established alfalfa is data growth begins in spring or at harvest of preceding crop; final harvest is data crop becomes dormant.

Table 17 .-- Soil Moisture

Crop	Effective Root Zone	Available Moisture	Usable Moisture
V	feet	inches	inches
Spring grains	3.0	7.8	3.9
Winter wheat	4.0	10.0	4.5
Dry beans	2.0	5.2	2.5
Sugar beets	3.0	7.8	3.5
Alfalfa	5.0	12.0	6.0
Weight crop distribution	3.5	8.9	4.3

In computating effective precipitation and accounting for soil moisture, the XCIR program requires maximum available water and an initial soil moisture content. The maximum available water for the crop distribution is 8.9 inches, and it was assumed for this study the initial soil moisture content was 4.3 inches.

Crop Coefficients.--Information for accessing computer programs and data files pertaining to evapotranspiration computations are included in the Reclamation "Technical Guideline for Estimating Agricultural Crop Water Requirements" (dated September 1983). One of these data files (CRPCOFM) contains crop coefficient data (based on data developed by Dr. Marvin Jensen, et al., in 1973) which is specifically formatted for use with the XCIR program. These crop coefficients were assumed applicable to the project area and were used in the monthly consumptive use computations.

Further, the computer program (PEAKWRN) used to compute the peak daily ET includes a subroutine with crop coefficient data. This data was compared with the respective coefficient data in the above file (CRPCOFM) for the five crops proposed to be grown, and no differences were noted.

XCIR Results and Comparison with Other Estimates. -- The XCIR results consist of estimated monthly reference (potential) ET, estimated monthly ET

for each crop and the weighted crop distribution, and the estimated crop irrigation requirements for the weighted crop distribution.

A monthly summary of the weighted crop evapotranspiration (ET) for the period 1965 through 1982 is presented in table 18. The average annual weighted crop irrigation requirements (ET minus effective precipitation) for the 18-year period is 20.68 inches per acre. Monthly and annual tabulations of the crop irrigation requirement are found in table 19.

The computed average annual weighted crop irrigation requirement was compared with data in two publications, the SCS "Irrigation Guide for Southern and Southeastern Idaho" (dated 1970) and Bulletin 516, "Consumptive Irrigation Requirements for Crops in Idaho," published by the University of Idaho in 1970. In both publications, crop irrigation requirements are developed for Rupert, Idaho. As shown in table 20, the computed annual weighted crop irrigation requirement compares reasonably with the data contained in the above two publications.

Table 18.--Monthly Evapotranspiration in Inch, Minidoka North Side Extension Weighted Crops

	m M OOK M		EXTENSI	O,	HTED CE									
EAR	JAN.	FEB.	MARCH	APRIL	MAY	JUNE	JULY	AUG.	SEPT.	OCT.	NOV.	DEC.	TOTAL	
		54	.04	1.13	3.07	5.74	7.56	3.90	2.00	1.26	0.00	0.00	24.71	
965	0.00	.01	.05	1.16	4.30	6.21	8.30	4.31	2.40	1.00	0.00	0.00	27.74	
966	0.00	.01		. 75	3.70	5.28	B. 05	4.72	2.41	. 99	0.00	0.00	25.95	
967	0.00	.01	.04	1.02	3.67	6.19	8.53	3.17	2.23	1.01	0.00	0.00	25.88	
968	0.00	.01	. 05	1.26	4.38	5.53	8.11	4.45	2.36		0.00	0.00	26.94	
969	0.00	0.00	.04		3.60	5.92	7.65	4.37	1.96	. 89	0.00	0.00	25.33	
970	0.00	.01	. 04	. 89 . 87	3.53	5.90	8.16	4.35	1.99	. 86	0.00	0.00	25.71	
1971	0.00	.01	.04		3.61	6.34	7.92	4.14	1.87	. 88	0.00	0.00	25.83	
972	0.00	.01	. 05	1.01	4.15	6.40	7.70	4.21	2.17	1.04	0.00	0.00	26.78	
1973	0.00	.01	.04	1.06	3.88	7.56	8.05	4.37	2.69	.97	0.00	0.00	28.68	
1974	0.00	.01	. 04	1.11		5.89	7.97	3.99	2.54	.91	0.00	0.00	25.02	
1975	0.00	0.00	. 04	. 67	3.01	6.19	7 B1	3.60	2.29	1.05	0.00	0.00	25.94	
1976	0.00	.01	.03	. 98	3.98	6.49	7.41	3.68	2.04	1.04	0.00	0.00	24.61	
1977	0.00	.01	.04	1.30	2.60		7.43	3.94	1.95	1.12	0.00	0.00	24.62	
1978	0.00	.01	. 05	.86	3.12	6.14		3.50	2.54	. 99	0.00	0.00	25.60	
1979	0.00	0.00	.04	. 99	3.69	6.34	7.51	3.68	1.91	. 99	0.00	0.00	22.82	
1980	0.00	.01	.04	1.05	2.89	5.25	7.00	4.77	2.48	.75	0.00	0.00	26,82	
1981	0.00	.01	. 05	1.10	3.04	6.28	8.34	4.37	1.96	. 88	.0.00	0.00	24.77	
1982	0.00	0.00	.04	. 92	3.49	6.06	7.05	4.37	1.50	. 60	.0.00	0.00		
MEANS	0.00	0.00	.04	1.01	3.54	6.10	7.81	4.08	2.21	. 97	0.00	0.00	25.76	25.7
MAX	0.00	.01	. 05	1.30	4.38	7.56	8.53	4.77	2.69	1.25	0.00	0.00		
MIN	0.00	0.00	.03	.67	2.60	5.25	7.00	3.17	1.87	. 75	0.00	0.00		
STDEV	0.00	.00	.01	. 16	. 50	. 52	. 43	. 43	. 25	. 12	0.00	0.00		
KC	0.00	.01	.02	. 28	. 60	.81	. 85	.54	, 46	.37	0.00	0.00		

Table 19.--Monthly Crop Irrigation Requirements, Minidoka North Side Extension, Elevation = 4250 Feet

MIN.	PCT. AW	ON REQUIS C END-OF TER HOLD	HTMOM-	100 1	00 100 11 IELD CAP		90 90	UES BASE 90 90			EMP AND (P AWC AT FIELD CAP
		FEB	MAR	APR	MAY	JUN	JUL	AUG	SEP	OCT	NOV	DEC	TOTAL
EAR	JAN 0.00	0.00	0.00	0.00	1, 16	5.28	7.22	3.12	1.46	1.26	0.00	0.00	19.50:
965 966	0.00	0.00	0.00	.93	3, 11	6.12	8.00	4.29	2.26	.75	0.00	0.00	25:46:
967	0.00	0.00	0.00	0.00	2 29	3 25	7.79	4.71	2.16	. 14	0.00	0.00	20.34:
968	0.00	0.00	0.00	. 62	1.94	5.35	8.53	. 55	1.97	. 73	0.00	0.00	19.71;
969	0.00	0.00	0.00	1.02	3.38	4.62	7.97	4.45	1.60	. 42	0.00	0.00	23.45:
970	0.00	0.00	0.00	. 22	1.73	4.49	6.67	4.33	1.03		0.00	0.00	18.83:
971	0.00	0.00	0.00	0.00	89	4.65	8.15	4.09	1.32	. 22	0.00	0.00	19.33:
972	0.00	0.00	0.00	. 70	2.43	5.53	7.91	3.65	1.04	0.00	0.00	0.00	21.46:
973	0.00	0.00	0.00	16	2.72	5.44	7,25	4.15	1,37	. 81	0.00	0.00	21.89:
974	0.00	0.00	0.00	. 48	2.86	7.34	7.B1	4.30	2.69	0.00	0.00	0.00	25,47:
975	0.00	0.00	0.00	0.00	0.00	5.33	7.57	3.94	2.52	0.00	0.00	0.00	19.36:
976	0.00	0.00	0.00	20	2.72	5.46	7.58	1.84	1.58	. 40	0.00	0.00	19.77:
977	0.00	0.00	0.00	1.17	.08	5.84	5.28	3.36	1.34	1.00	6.00	0.00	19.07:
978	0.00	0.00	0.00	0.00	1.17	6.01	7.32	3.74	0.00	1.00	0.00	0.00	19.25:
979	0.00	0.00	0.00	.70	2.14	5.93	7.41	2.80	2.52	. 24	0.00	0.00	21.75:
1980	0.00	0.00	0.00	. 26	0.00	4.05	6.84	3.58	, 39	. 68	0.00	0.00	15.80:
1981	0.00	0.00	0.00	0.00	1,47	6.03	6.33	4.77	2.17	0.00	0.00	0.00	22.76:
982	0.00	0.00	0.00	.07	2.25	5.59	6.42	3.98	. 65	0.00	0.00	0.00	18.97:
AVE	0.00	0.00	0.00	. 36	t . BO	5.35	7.50	3.86	1.56	. 45	0.00	0.00	20.68

Table 20.--Comparison of Results, Annual Crop Irrigation Requirement for the Weighted Crop Distribution

Item	XCIR Results	SCS Irrigation Guide	Bulletin 516 ¹ /
Consumptive irrigation requirement (inches per acre)	20.68	20.29	21.42
Percent of XCIR results (percent)	₩	98.1	103.6

^{1/} Assumes that 8 out of every 10 years the crop irrigation requirement will be met.

The historic 20-year average (1963 through 1982) annual project farm delivery for the A&B Irrigation District was 3.06 acre-feet per acre (36.72 inches). Assuming an average annual consumptive irrigation requirement of 1.72 acre-feet per acre (20.68 inches), the average annual onfarm irrigation efficiency would be 56 percent.

Farm Irrigation Efficiency

Initially, the majority of the project was gravity irrigated, but with the increased efficiency associated with sprinkler irrigation, approximately 30 percent of the project land has been converted to sprinkler irrigation (1983 crop census). Sprinkler irrigation properly adjusted to the soil infiltration rate is generally more efficient than gravity methods. The topography of the Minidoka North Side Extension is well suited for sprinkler irrigation, having average slopes of 2 to 4 percent. It was assumed that all newly irrigated lands in this project would be sprinkler irrigated and have an irrigation application efficiency of 70 percent.

Redivertible Return Flows

The higher efficiency of sprinkler irrigation systems makes reuse of onfarm runoff uneconomical. The irrigators in the district exhibit good farming practices, and it was assumed the surface return flows from the new land would be insignificant and uneconomical to reuse. However, the aggregation of multiple farm runoff from the existing system may provide a portion of the irrigation demands to lands near the major drainageways within the project. The accumulated irrigation return flow and its reuse was discussed in the water supply section of this appendix.

Conveyance System Losses

Water losses due to seepage and operational spills are termed conveyance losses. Monthly seepage losses were computed as a percentage of the annual diversion requirements using the district's historical records as the data base (period selected was 1963 through 1982). With project conditions, the Extension land in Unit A of the district will be served by enlarging existing canals (earth-lined), whereas new concrete-lined canals will be constructed to serve the new lands in Unit B. Therefore, diversion requirements were

computed for two conditions: (1) those lands served by the enlarged existing system (Unit A) and (2) those lands served by new concrete lined canals (Unit B).

The seepage computations were based on the channel seepage equation (assumes free drainage conditions) presented in the "Drainage Manual" published by the Bureau of Reclamation in 1978. To account for the additional seepage losses attributable to the new lands in Unit A, a typical cross section was selected for computations. The seepage loss was estimated for the existing section based on the district's records from 1963 through 1982, and then a new seepage loss was estimated for the enlarged canal section. The percent change in seepage losses associated with enlarging the selected canal section (required to meet the demands of the new lands in Unit A) was assumed to represent all enlarged canal sections and were attributed to the newly irrigated lands.

The A&B Irrigation District's records were examined from 1963 through 1982 for Unit A, and the average annual seepage loss for this 20-year period was 19 percent of the average annual diversion requirement. The annual seepage loss associated with the new lands served in Unit A (enlarged canal section) was estimated at 15 percent of the annual diversion requirement for the new lands.

Operational spills result from less than 100 percent efficient operation of the irrigation system. Unavoidable losses through waterways occur due to the improper timing of releases and diversions. After reviewing the historic records for the A&B Irrigation District, it was assumed operational losses for the new lands served in the Unit A would be 6 percent of the annual diversion requirements for the new lands.

Seepage losses and operational spills for the Extension land served in Unit B should be minimal. The new canals are concrete lined and very short in length. Although the existing canals in Unit B are short in length, the losses associated with the existing system will be slightly higher than the losses in the new system because the existing canals are not lined. The 20-year (1963-82) average annual conveyance losses (seepage plus operational) in Unit B were 8 percent of the average annual ground water pumped.

Accounting for the concrete lining, it was assumed the total annual conveyance losses for the new lands would be 5 percent of the annual ground water pumped to the proposed new lands in Unit B.

Leaching Requirement

No additional water for leaching of salts would be required for any of the Extension lands. The present salinity level in the soils is very low. Irrigation water applied from either the Snake River or ground water would be of high quality.

Diversion Requirements

Summaries of the diversion requirements for the new land in Units A and B are presented in tables 21 and 22, respectively.

Table 21.--Diversion Requirements, New Lands Served by Unit A (inches per acre)

	Weighted Crop Irrigation	Farm	Conveya	Diversion	
Month	Requirement	Delivery	Seepage	Operational	Requirement
April	0.36	0.51 2.57	0.10 0.49	0.04 0.19	0.65 3.25
May June	1.80 5.35 7.50	7.64 10.71	1.45 2.03	0.58 0.81	9.67 13.55
July August September	3.66 1.56	5.23 2.23	0.99 0.42	0.40 0.17	6.62 2.82
October	0.45	0.64	0.12	0.05	<u>0.81</u>
Total	20.68	29.53	5.60	2.24	37.37

Table 22.--Diversion Requirements, New Lands Served by Unit B (inches per acre)

Month	Weighted Crop Irrigation Requirement	Farm Delivery	Conveyance Losses—	Diversion Requirements
April	0.36	0.51	0.03	0.54
May	1.80	2.57	0.14	2.71
June	5.35	7.64	0.40	8.04
July	7.50	10.71	0.56	11.27
August	3.66	5.23	0.28	5.51
September	1.56	2.23	0.12	2.35
October	0.45	0.64	0.03	0.67
Total	20.68	29.53	1.56	31.09

^{1/} Represents operational spills plus seepage losses

The plan would serve the Extension lands from three basic sources:

(1) Snake River pumping and the use of existing North Side Pumping Division

(A&B Irrigation District) facilities, (2) the use of water from existing A&B

Irrigation District drains, and (3) ground-water pumping to serve the lands in

Unit B. The total area to be irrigated from each source and the annual diversion requirements are presented in table 23.

Table 23.--Annual Diversion Requirements by Source

Source	Extension Area	Annual Diversion Requirement
Source	acres	acre-feet
Snake River supplies from North Side Pumping Division	3,950	12,300
Drainwater from existing North Side Pumping Division drains Ground water ²⁷	580 5,680	$\frac{1,500}{14,700}$
Total	10,210	28,500

^{1/} Drainwater would need to be augmented with ground-water pumping. For Area 12, existing project wells would be required to augment both early and late season drainwater flows. A new well would be required to augment H Main Drain flows for much of the irrigation season to serve lands in Area 11.

2/ Does not include 240 acres of square-up lands scattered throughout Unit B. The annual diversion requirement for this land is 620 acre-feet and will be provided from existing project wells in Unit B.

Section II--Peak Delivery Requirement

The frequency of irrigation is determined by the amount of usable soil moisture available for crop consumptive use. This study assumes no net withdrawal of usable soil moisture during the design peak consumptive use period. As a result, the number of days in the design peak consumptive use period equals the frequency of irrigation at peaking as determined by the usable soil moisture. This assumption is based on consumptive use data that indicate there can be several peak periods of about the same magnitude, and these periods can essentially occur back-to-back with little opportunity to replace any net withdrawal of soil moisture.

Design Rate of Evapotranspiration

The design evapotranspiration rate was computed by use of the Engineering & Research Center computer program PEAKWRN (utilizing the Jensen-Haise procedure). First the program estimates daily ET, and then it identifies the peak daily moving mean evapotranspiration for a selected interval of days for each year of the period of record. The design rate for the selected interval is then defined as the mean annual peak daily evapotranspiration plus one standard deviation. The required delivery of soil moisture to the plants during the interval is then equal to the product of interval days and the design rate. The selected interval for a given crop or a weighted crop distribution equals the number of days between irrigations as determined by the usable soil moisture. This requirement assumes a zero net withdrawal of soil moisture during the peak consumptive use period. It is also assumed that the water consumptively used during the peak period will be entirely replaced by irrigation and that there will be zero effective precipitation.

Since the irrigation interval corresponding to the usable soil moisture cannot be known until the design rate is known, it was necessary to compute

design rates for several intervals and plot them against usable soil moisture. Using daily temperatures and solar radiation data, daily moving team evapotranspiration values were computed for 1-, 3-, 7-, 10-, 15-, 20-, and 30-day intervals for each crop and crop distribution. Design rates of evapotranspiration for the project area were computed by adding one standard deviation to each yearly mean peak daily evapotranspiration.

Data Input to PEAKWRN

The basic data required in the computations of peak delivery requirements include project crop distribution, soil moisture data, daily temperature data, daily solar radiation, and crop coefficients. These parameters are thoroughly discussed in "Section I--Monthly Diversion Requirement;" however, one difference exists. The PEAKWRN program requires daily climatological data, whereas the XCIR required monthly climatological data. The availability of daily climatological data differs slightly from the monthly data used in section I in that the daily data is available from 1965 through 1980 (for the April through October period), whereas the monthly data is available from 1965 through 1982. Therefore, the period of record (1965-80) used in the peak delivery requirements will be 2 years shorter than the period (1965-82) used in the monthly diversion requirements.

PEAKWRN Results

A summary of the peak daily ET results are shown in table 24. These results are based on a mean project elevation of 4250 feet. The mean peak occurs in July for beans, sugar beets, alfalfa, and the weighted crop distribution, whereas, the mean peak for winter wheat occurs in June. The small grains (barley and spring wheat) mean peak occurs in June or July depending on the time interval.

Table 24.--Design Rate of Peak Evapotranspiration

Crop	Peak ET	Month of Occurrence
	(inches/day)	0
Spring grain	.334	June-July
Winter wheat	.303	June
Beans	.337	July
Sugar beets	.298	July
Alfalfa	.334	Ju1y
Weighted crop distribution	.304	July

The peak daily ET for several time intervals were plotted against usable soil moisture for each crop and the weighted crop distribution. The usable soil moisture for each crop and the weighted crop distribution are discussed in section I and presented in table 15. Entering the above curves with the appropriate usable soil moisture from table 15, the design peak evapotranspiration rates were computed for each crop as presented in table 24. Comparison with Other Estimates

Peak daily ET at Rupert, Idaho, is published in the SCS "Irrigation Guide for South and Southeastern Idaho" (dated 1970) by soil type and associated slopes. A portneuf silt loam with a slope of 2 percent was selected to represent the project area. The peak daily ET for each crop and the weighted crop distribution from the above publication are listed in table 25. Table 26 compares the PEAKWRN results and the farm delivery requirement assuming a 70-percent irrigation efficiency.

Table 25.--Peak Daily Evapotranspiration (Mean Annual Plus One Standard Deviation), 1965-80

TEMPERATURE & SOLAR						BENLET		
LOCATION MINIDOKA	NORTH S	IDE EXTE	NSION	ELEVATI	ON 425	O. FT		
			LENGTH	OF PERS	OD IN DA	vs		
CROP	1	3	5	7	10	15	20	30
10 10000								
POTENTIAL ET			207	250	. 351	.344	. 339	. 33
PEAK ET (INCHES PER DAY)	. 394	.382	. 367	. 359	3.51	5.16	6.77	9.93
PEAK ET X DAYS (INCHES)	. 39	1.15	1.83	2.52	3.51	3.10	9.77	. 3.33
SMALL GRAINS				250	244	.332	. 325	. 31
PEAK ET (INCHES PER DAY)	. 392	. 38 1	. 365	. 356	. 344	4.98	6.49	9.30
PEAK ET X DAYS (INCHES)	. 39	1.14	1.82	2.49	3.44	4.90	0.45	9.30
WINTER WHEAT					0.40	205	. 294	. 27
PEAK ET (INCHES PER DAY)	. 367	.348	. 334	. 327	.318	.305	5.87	8.28
PEAK ET X DAYS (INCHES)	. 37	1.04	1.67	2.29	3.18	4.57	3.87	8.20
BEANS		1920 002				. 323	. 315	. 29
PEAK ET (INCHES PER DAY)	. 358	. 346	. 342	. 337	. 331	4.85	6.31	8.90
PEAK ET X DAYS (INCHES)	. 36	1.04	1.71	2.36	3.31	4.63	0.31	0. SU
SUGAR BEETS	2000			040	.311	. 304	. 298	. 29
PEAK ET (INCHES PER DAY)	. 344	. 335	. 325	.318	3.11	4.56	5.96	8.72
PEAK ET X DAYS (INCHES)	. 34	1.00	1.62	2.23	3.11	4,50	3.50	0.72
ALFALFA			122		0.40	200	320	. 32
PEAK ET (INCHES PER DAY)	. 386	.374	. 364	. 356	.348	. 338	. 332	9.64
PEAK ET X DAYS (INCHES)	. 39	1.12	1.82	2.49	3.48	5.08	6.64	3.64
MINIDOKA N.S. EXT.								
WT 31 8 8 26 27 0 0 0			Quinter and	772222707				
PEAK ET (INCHES PER DAY)	. 35 1	.340	. 328	. 321	. 311	. 304	. 295	. 28
PEAK ET X DAYS (INCHES)	. 35	1.02	1.64	2.25	3.11	4.56	5.90	8.52

Table 26.--Comparison of Peak Evapotranspiration and Net Farm Deliveries

	Peak Evapotran	spiration
Crop	SCS Publication	PEAKWRN
0,00	(inches/day)	(inches/day
Grains Beans	0.23 0.30 0.28	$0.32\frac{1}{}^{\prime}$ 0.34 0.29
Sugar beets Alfalfa	0.29	0.33
Weighted crop distribution	0.265	0.304
Farm delivery for the weighted crop distribution	0.379	0.434

 $[\]underline{1}$ / Average of winter wheat and spring grains

The PEAKWRN results are consistently higher for beans, alfalfa, and grains. The pertinent data associated with peak daily consumptive use computations was reviewed, and the PEAKWRN results appeared to be reasonable and within acceptable limits.

Peak farm delivery data appears in the publication "Sprinkler Irrigation in the Pacific Northwest, A Troubleshooter's Guide." In this publication a range was developed for peak farm delivery in Idaho assuming sprinkler irrigation with an operation of 24 hours per day 7 days per week with an irrigation efficiency of 70 percent. The range developed for the project area was 0.34 to 0.45 inches per day (peak ET of 0.24 to 0.31 inches per day). The peak daily ET computed in PEAKWRN and peak daily ET published in the SCS guide fall within this range.

The existing project was designed and constructed on a peak farm delivery of 0.357 inches per day, which compares with the SCS data. In a letter from the A&B Irrigation District to the Bureau of Reclamation dated May 24, 1984, the district views the development of the Extension lands as a completion of the original project and recommends a peak farm delivery no greater than 0.357 inch per day. Assuming 70-percent irrigation efficiency, this would be a peak ET of 0.25 inches per day. The newly irrigated lands will be an extension of the project; therefore, it was assumed the irrigators would continue to diversify their crops. The peak rate of 0.357 inches per day is recommended for use to develop peak delivery requirements. This rate represents the capacity required at the turnouts and is not inclusive of conveyance losses in the laterals and the canals.

CHAPTER 6--WATER UTILIZATION

Surface-water utilization for the Minidoka North Side Pumping Division Extension project was simulated by using the upper Snake River digital model based on the period 1928 through 1978. The Snake River model has been used in previous planning studies and subsequently was used in this study. There were no significant revisions to the model during this study; however, the data base used in the previous study was expanded to include an additional 10 years.

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The model allows one to measure the impacts of any proposal on the upper Snake River system. The digital model operates the Snake River system of reservoirs and, whenever possible, provides a water supply to the users using natural flows, storage, and the rental pool. These three sources are utilized by a project or canal company in a specific order similar to the existing practices of the users. The reservoir contents and riverflow resulting under the proposed project conditions for the 51-year period examined can be seen on table 27 (units are in 100 acre-feet) at the end of this chapter.

Certain assumptions have been made which affect the values produced in the operation study. The most significant assumption involves the amount of storage space arbitrarily assigned to the Salmon Falls project which affects the yield from Ririe Reservoir. Since no final assignments have been made of Ririe storage at the time of this appendix, the ownership assignments will likely shift as the contracts are completed for the unassigned storage space.

The majority of the canals in the upper Snake River system do not have diversion records spanning the 51-year period examined (one of which is the A&B Irrigation District). To measure any impacts (such as the proposed project) on the Snake River system for an extended period, monthly diversion demands were estimated for the canals with limited records, and these estimates were based on selected canals (key canals) in the upper Snake River system which have long-term diversion records and have maintained the same irrigated area throughout the 51-year period. The monthly diversion demands computed for the existing and the new Unit A lands were based on the Twin Falls South Side Canal. These estimated demands differ slightly from historic records; however, operation model runs for the existing system indicate the total demand on the Snake River system is representative of actual practices. Further, natural flow and storage diversions were computed for the existing

Unit A lands using the model, and as illustrated in table 28, these results compare favorably with the historic operations for the 16-year period examined (the historic records and the model results overlap from 1963 through 1978).

Table 28.--Comparison of Results for Existing Unit A Lands (1963 through 1978)

	Natural Flow Diversions	Storage Diversions
	acre-feet	acre-feet
Historical	21,740	34,890
Historical Model—	22,430	34,200

^{1/} Assumes no shortages. The district's records indicate they did not experience a shortage during this period.

To effectively evaluate the impacts of the proposed new Unit A lands on the district's surface-water supply and considering the complexity of the model, the monthly irrigation demands of the new Unit A lands and the existing Unit A lands were combined and not treated as separate diversions. The irrigation diversions of the new lands represent about 18.2 percent of the combined irrigation demands. A summary of the model results with the combined irrigation demands are presented in table 29 (at end of this chapter).

The main intent of using the digital model is to ascertain the impacts on the district's surface-water rights and, more importantly, to assure an adequate water supply during dry periods. Under Idaho law, the district can only divert natural flow from the Snake River to the existing Unit A lands, however, due to constraints in the digital model, natural flow water was diverted to the new Unit A lands when it was available. The results presented in table 29 were examined, and this inconsistency had negligible effects during dry periods but provides a significant part of the diversion requirements of the new Unit A lands in wet years. Even though this A&B 617

inconsistency has minimal effects during dry periods, the water supply was recomputed on an annual basis, assuming no natural flow diversions to the new lands. These results are presented in table 30, and selected years are summarized in table 31.

As previously discussed, the main factor which enables the district to provide a dependable water supply to the new land is the refill capabilities of American Falls. Under project conditions, the district received their full American Falls allocation in 49 out of the 51 years examined. In the 2 years (1934, 1960) the district did not receive a full allocation, they received 90 percent of the full allocation in 1 year and 98 percent in the other.

In 47 years of the 51-year period examined, part or all of the storage water diverted to the new Unit A lands came from American Falls Reservoir. The average American Falls storage diverted to the new lands over the 51-year period was 9,666 acre-feet or about 80 percent of the total storage water diverted to these lands. The model results indicate a dependable water supply can be provided to the proposed new Unit A lands under the district existing storage water rights without producing any significant impacts on the existing Unit A lands. The shortages which occur are voluntary in nature and are well within the accepted criteria adopted by the Bureau of Reclamation. A full discussion of the voluntary shortages is discussed later in this chapter.

Table 30.--Annual Summary, North Side Pumping Division Operation Study

Water	Incipation	Value	Water Natura	Supply Storage	American Falls	Palisades
Year	Irrigation Demand	Shortage2/	Flow-3/	Water	Storage Used	Storage Used_
				-feet		
1928	68,660	0	34,807	33,853	33,853	0
1929	67,060	0	26,269	40 791	40,791	0
1930	67,060 63,380	0	14,186	49,194	46,826	2,368
1931	62,490	0	10,024	52,466	46,826	5,640
1932	63,340	0	20,269	43,071	43.071	0
1933	69,240	0	21,970	47,270	46,826	444
1934	57,736	-14,511	3,690	39,535	39,535	0
1935	63,008	-21,336	18,900	22,772	22,772	0
1936	67,660	0	21,570	45,990	45,990	0
1937	65,700	0	16,955	48,745	46,826	1,919
1938	65,190	0	26,098	39,092	39,092	0
1939	68,910	0	12,000	56,910	46,826	10,084
1940	66,200	0	10,559	55,641	46,826	8,815
1941	64,090	0	5,638	58,452	46,826	11,626
1942	64,290	0	11,319	52-971	46,826	6,145
1943	66,220	0	32,842	33,378	33,378	0
1944	62,110	0	14,729	47,381	46,826	555
1945	65,070	0	25,238	39,832	39,832	0
1946	65.910	0	27,488	38,422	38,422	0
1947	66,370	.0	27,367	39,003	39,003	0
1948	67,640	0	31,270	36.370	36,370	0
1949	67,410	0	21,410	46,000	46,000	0
1950	68,350	0	30,612	37,738	37,738	0
1951	68,170	0	26,596	41.574	41,574	0
1952	72,100	0	28,156	43,944	43,944	0
1953	70,240	0	20,724	49,516	46,826	2,690
1954	69,780	0	27,121	42,559	42,659	0
1955	68,360	0	22,249	46,111	46,111	0
1956	68,800	0	23,094	45,706	45,706	0
1957	67,710	0	24,926	42,784	42,784	0
1958	68,010	0	19,044	48,966	46,826	2,140
1959	66,350	0	28,744	37,606	37,606	0
1960	68,950	0	2,190	66,760	46,826	19,934
1961	65.410	0	7,630	57,780	45,687	12,093
1962	70,360	0	24.844	45,516	45,516	0
1963	69,260	0	23,486	45,774	45,774	0
1964	71,310	0	28,956	42,354	42,354	0
1965	71.610	0	28,553	43,057	43,057	0
1966	72,140	0	4,530	67,610	46,826	20,784
1967	73,320	0	25,988	47,332	46,826	506
1968	70.820	0	12,820	58,000	46,826	11,174
1969	72,240	0	9,598	62,642	46,826	15,816
1970	68,820	0	27,140	41,680	41,680	0
1971	70,920	0	28,826	42,094	42,094	0
1972	70,470	0	28,533	41,937	41,937	0
1973	68,450	0	11,782	56,668	46,826	9,842
1974	69,540	0	27,750	41,790	41,790	0
1975	66,090	0	28,952	37,138	37,138	0
1976	69,760	0	29,363	40,397	40,397	0
1977	60,346	-15,636	6,642	38,068	38,068	0
1978	63,080	0	35,984	27,096	27,096	0
Average Average without	67,421	-1,010	21,167	45,244	42,448 <u>1</u> /	2,796
shortages 2/	67,421	0	21,167	46,253	43,089	3,164

^{1/} This value represents the combined irrigation demands of the existing Unit A land (81.8 percent) and the proposed new Unit A lands (18.2 percent).
2/ Irrigation shortages are voluntary in nature. Model results indicate there was sufficient storage in the 3 years to meet these shortages.
3/ Natural flow is only diverted to the existing Unit A lands

Table 31.--Summary of Results in Acre-feet, 1928 through 1978

		Existin	g Unit A La	nd			Extensio	n Unit A L		Constant of
	(A		Natural		Diversions			Natural		Diversions
Event 1/	Irrigation Requirement	Shortage ^{2/}	Flow Diversion	American Falls	Palisades 3/	Irrigation Requirement	Shortage 1/	Flow Diversion	American Falls	Palisades3
Wet year (1943) Dry year (1934)	54,141	v	32,842	21,299	0	12,079	0	0	12,079	U
With voluntary shortages	47,205	-11.864	3,690	31.651	0	10.531	-2.647	0	7,884	U
Without voluntary shortages	47,205	0	3,690	42.131	1,384	10,531	U	0	0	10,531
Typical year (1958) Average 1928-78	55,605	ō	19.044	36,561	0	12,405	O	0	10,265	2,140
With voluntary shortages	55,123	-826	21,167	32.782	348	12,298	-184	0	9,666	2,448
Without voluntary shortages	55,123	0	21.167	33,956	348	12,298	0	0	9,481	2.816

1/ Years selected were based on the historic date from 1928 through 1978 for the Snake River at Neise
2/ Shortages are voluntary in nature and the digital model results indicate the district has siffucient storage space to adequately meet all of

7 Prior to 1978, a conveyance loss was charged to Palisades storage when it was utilized. This has been discontinued and these values reflect

Rental Pool

One main objective of the digital model was to maximize water utilization on the upper Snake River system. Projects and canal companies with the ability to contribute water to the rental pool were examined, and an arbitrary annual contribution was established for each entity. This contribution represents the maximum amount of storage water which can be contributed to the rental pool without producing significant impacts on the existing water supply. For the A&B Irrigation District, it was assumed all Palisades storage water in excess of 32,100 acre-feet (district's Palisades storage right is 90,800 acre-feet) would be made available to the rent pool. This rental pool limit value has remained the same in this study as it has throughout all previous planning project studies.

In the digital model, the entities storage water is reduced to the established rental pool limit during the month in which storage water is allocated to all users. Then, at the termination of each irrigation season, the unused portion of the rental pool is credited back to each contributing entity.

Presently, the A&B Irrigation District contributes 50,000 acre-feet of Palisades storage to the rent pool annually. An examination of the model results indicates the district could continue to contribute to the rent pool A&B 620

and provide a dependable water supply to the proposed new Unit A lands. However, it may be prudent for the district to modify their contribution during dry periods.

Shortages

There are three dry periods during the 51-year period examined, occurring in 1934, 1935, and 1977. In these years, voluntary water shortages were assigned to each project or canal company and were implemented to maximize water utilization and carryover storage. The shortages are input as a percentage of the monthly irrigation demands, and it was assumed the same percentage would apply to both the new and the existing Unit A lands. It should be emphasized that these shortages are only voluntary in nature, and the digital model results (table 29) indicated the district had sufficient storage to adequately meet all of the irrigation demands during the 3 years where voluntary shortages were implemented. Further, the district contributed storage water to the rental pool in all 3 years, and based on the model results the district should not experience water shortages under the Recommended Plan. During dry periods the district could modify or eliminate their contribution to the rental pool which would assure a full water supply to the existing system and the proposed project.

Effects on the Upper Snake River System

Providing full irrigation service to lands within the A&B Irrigation District would have a negligible impact on upper Snake River system surface-water supplies. The Recommended Plan includes the use of Snake River water to provide full irrigation service to 3,950 acres $\frac{1}{}$ of land located in

^{1/} Includes about 3,140 acres of new irrigated land and 810 acres of existing irrigated land requiring a replacement water supply.

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Unit A of the A&B Irrigation District. Ground water would be pumped from the Snake Plain aquifer to provide full irrigation service to 5,680 acres of land in Unit B. As previously discussed, surface return flow under the Recommended Plan would be insignificant; however, irrigation losses due to deep percolation will ultimately end up in the Snake Plain aquifer.

Pumping from the Snake Plain aquifer does not approach recharge. Present pumping is estimated at 1.4 million acre-feet per year, and recharge is estimated at about 8 million acre-feet per year. The estimated net average annual ground-water requirement for the new Unit B lands is 13,520 acre-feet, about 1 percent of the present Snake Plain aquifer pumping. Under the Recommended Plan, the aquifer should accommodate the proposed ground-water pumping without showing any significant declines. Further, the effects of the additional pumping will not be identifiable at Thousand Springs.

The estimated annual surface-water requirement for the new Unit A land is 12,300 acre-feet and represents less than 0.5 percent of the 2.9 million acre-feet of combined active storage in these two reservoirs. Such a small percentage of the total storage could be released without measurably affecting water surface elevations at the two reservoirs.

Assuming an average annual diversion requirement of 12,300 acre-feet, the maximum increase in Snake River flows below American Falls Reservoir would be about 60 ft³/s during the peak irrigation period. The average peak irrigation flow below American Falls is currently about 12,200 ft³/s; with the Extension project, this flow would increase by about 0.5 percent. This slight increase in flow is not significant. Comparing an average annual diversion requirement of 12,300 acre-feet to an average annual diversion of 8.2 million acre-feet (period of record 1928 through 1981) for the upper Snake River system, there would be less than a 0.2-percent change in total diversion requirements. Dry

and wet periods were also examined for these two cases, and the relative impacts on the upper Snake River system remained negligible.

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Table 27.--Column Description, Snake River Flow and Reservoir Contents

Column	Description
WYR	Water year of operation study
MO	Month of operation study
FCSPSN	Flood control space required on the main stem of the Snake
1 031 311	River (hundreds of acre-feet)
FRCSTH	Heise runoff forecast from the beginning of the given month
TROSTIT	to July 31 (hundreds of acre-feet)
SYSCON SNAKE	The summation of the end-of-month contents of Jackson Lake, Palisades, and American Falls Reservoirs (hundreds of acre-feet)
SYSCON H. FORK	The summation of the end-of-month contents of Island Park Reservoir, Grassy Lake, and Henrys Lake (hundreds of acre-feet)
RENT POOL	Amount of water in the system which is available to be rented out to canal companies in need of supplemental water (acre-feet)
XCUT CANAL	Water diverted by the Crosscut Canal into the Teton River (acre-feet)
GAIN USED BY	Reach gains and return flows diverted to the Minidoka and
MINDKA BURLEY	Burley Irrigation Districts. These gains and return flow
1121121111 2211221	occur in reaches 108 and 109 of the Snake River (acre-feet)
TOTAL XCUT	Total water diverted into the Crosscut Canal from the Henrys Fork of the Snake River and the Falls River (acre-feet)
H.FORK SMSHRT	The summation of irrigation shortage incurred by the canals
H.FUKK SHSHKI	in the Henrys Fork of the Snake River (acre-feet)
JACKSON LAKE	(hundreds of acre-feet)
INFLOW	Inflow to Jackson Lake
EOM	End-of-month content of Jackson Lake
OUTFLO	Outflow from Jackson Lake
PALISADES	(hundreds of acre-feet)
INFLOW	Inflow to Palisades
EOM	End-of-month content of Palisades
OUTFLO	Outflow from Palisades
HEISE FLOW	Snake River flow near Heise (hundreds of acre-feet)
HENRYS LAKE	(hundreds of acre-feet)
EOM	End-of-month content of Henrys Lake
OUTFLO	Outflow from Henrys Lake
ISLAND PARK	(hundreds of acre-feet)
INFLOW	Inflow to Island Park
SEEPAGE	Island Park seepage loss
EOM	End-of-month content of Island Park
OUTFLO	Outflow from Island Park
H.FORK ASHTON	Flow in the Henrys Fork of the Snake River near Ashton
Har Old Monton	(hundreds of acre-feet)
GRASSY LAKE	(hundreds of acre-feet)
EOM	End-of-month content of Grassy Lake
OUTFLO	Outflow from Grassy Lake

Table 27.--Continued

Column	Description
FL RIV CHESTER	Fall River flow near Chester (hundreds of acre-feet)
H.FORK STANTY	Flow in the Henrys Fork at the Snake River near St. Anthony (hundreds of acre-feet)
TT RIV STANTY	Teton River flow near St. Anthony (hundreds of acre-feet)
H.FORK REXBRG	Flow in the Henrys Fork of the Snake River near Rexburg (hundreds of acre-feet)
SHELLY FLOW	Snake River flow near Shelly (hundred of acre-feet)
BLFT FLOW	Flow in the Snake River near Blackfoot (hundred of
AMERICAN FALLS	acre-feet) (hundreds of acre-feet)
INFLOW	Inflow to American Falls
EOM	End-of-month content of American Falls
OUTFLO	Outflow from American Falls
LAKE WALCOTT	(hundreds of acre-feet)
INFLOW	Inflow to Lake Walcott
EOM	End-of-month content of Lake Walcott
OUTFLO	Outflow from Lake Walcott
MILNER FLOW	Snake River flow past Milner Dam (hundreds of acre-feet)
RIRIE	(hundreds of acre-feet)
INFLOW	Inflow to Ririe
EOM	End-of-month content of Ririe
OUTFLO	Outflow from Ririe
DIVRTD	Option not used
DIVRTD	Ririe water diverted to Progressive Irrigation District's old lands (acre-feet)
PUMP	Option not used
FLOW148	Flow in reach 148 of the Snake River (hundreds of acre-feet)
OPCODES	Values establishing operating limits for the system. The codes set operation limits for the eight reservoirs presented, both in content and downstream reaches.

RIGHTS			
S 1939	H. FORK SMSHRT	-623.	
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IDDEL 1928-78 W/O SALMON FALLS, INCLUDES 1939 RIGHTS

SNAKE RIVER DIGITAL MODEL

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1939	H. FORK SMSHRT	i	ö	Ö	Ö	ó	Ö	Ċ	ċ	; c	; (!	16649	1994.	ċ	28643.	Ö	o ·	o	0	o ·	o i	o (0	0	٠ د	٠ د	0	•	0	0 (0 (> (>	•	> C	3 (5 (•	0	0	0	
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MODEL	SYSCON	T. FORK	6	730			1100	1346.	1522.	1614.	1657.	1586.3			831.	14347.	829.	963	978	1200	9	1591	1690	1724	1861	1843.	1662.	1459.	17202.	1483	1518	1525.	1748.	1755.	1940.	2134.	2176	2036	1702	1449	1477	20943	
DIGITAL M		SNAKE	10267				27914.	306908.	34056.	36367.	36919.	38859.	30934	22569.	18459.	345442.	17652.	20351.	24146	27640.	30731.	34453	37881.	39578	39839	37627	29569	25132.	20565.364598.	24115.	26355.	29361.	33257.	35882.	37047.	38731.	39774	38027.	30557.	22010	18394	15780,373511,	
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		H OUTFLO ASI	237.							446	126	461.	446.		3052.	234.		246.	31.	. 28	. 19	248	743.	237.	461.	461,	446.	000		288.	250.	246	91.	222.	61.	265.	504	446.	573	512	207.	3605	,
		PARK EOM OUT	479.	589.	778.	964.	1131.	1282.		1204		0.00	699.		11925.	692.	803.	781.	. 196	1132.	1288.	1350.	1350.	1350.	1253.	1113.	917.	1000	12995.	907.	907.	878	1067.	1039.	1191.	1350.	1350.	1166.	826.	649.	689.	42020	16060
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	PARK	681.	770.	946.	1127.	1289.	1350.	1328.	7201.	, oo	875	j	11651.	866.	983.	947.	1120.	1079.	1204.	1350.	1259.	1350.	856.	798.	1082	12894.	1071.	1071.	1057.	1011.	.696	1101.	1350.	1350.	1350.	1137.	908	776.	13153.
RIGHTS	ISLAND	ó	ö	o.	o o	ö	o o	o (o ·	o c	5	ö	o.	ö	o.	ö	o ·	ö	ö	o O	o.	o.	Ö	o	ò	ò	0	0	o	0	0	Ö	o.	o	o	Ö	o.	0
1939	FLOW SE	281.	220.	207.	210.	223.	382.	462.	350.	263.	332.		3502.	270.	234.	210.	204	181.	187.	290.	392.	334.	251.	333	318.	3204.	247	218.	232	200	180	194	409.	637.	584.	344.	330.	332.	3907.
INCLUDES	S LAKE DUTFLO INFLOW	ö	o c	ö	o.	ö	ö	တ	36.	ö	61.		165.	0	o	o o	o O	o.	ö	œ	22.	99	ó	61.	.09	184	Ö	c	ċ	ó	; -	6	43.	53.	78.	35.	61	.09	350.
FALLS, I	HENRYS EOM DU	664.	7.16	742.	768.	794.	821.	843.	843.	838.	759.	2	9191.	727.	745.	765.	786.	808	830.	843.	843.	843.	841.	791.	746.	9568.	763.	780	. 018		. 64	843.	843.	843.	843.	843.	783.	735.	9759.
SALMON F	HE I SE FLOW	2069.	1216.	1093	1030.	830.	1331.	6144.	6671.	5973.	4927.	.6797	35206.	2052.	1222.	1088.	1085.	975.	786.	723.	6299.	6117.	6060.	4861.	4413.	35682.	2110	1250		1001	710	1081	4007	6206.	9587.	6198.	5960.	3134.	42495.
0/M	TFLO	1845.	1041.	922	863.	639.	1183.	5694.	6286.	5779.	4716.	7047	32532.	1845.	1041.	922.	922.	833.	615.	595.	5856.	5687.	5816.	4551.	4161.	32845.	1845					917	3773.	5808	9014	5865.	5697.	2929.	39290.
1928-78	PALISADES EOM OUTFLO	1061.	1421.	17665.	12285	12927.	14000.	14000.	14000.	11924.	10382.	10650.	.146592.	10433.	10843.	11249.	11398.	11506.	12110.	13545.	14000.	14000.	12724.	12141.	11313.	58.145261.	1 1 1 2 2 2	1000	12076	19900	10000	14000	14000	14000.	13985	13816.	11789.	11955.	289.157466.
MODEL	3	1761.	1401.					5742.	6354.	3777.	3235.	2948.	32357.1	1650.	1452.	1328.	1071.	941.	1219.	2061.	6328	5755.	4615.	4035	3374.	33858.	8000	. 700	1104		1000	1221	. HORE.	5857	9066	5772.	3739.	3136.	40289.
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RIVER DIGITAL	JACKSON LAKE	5128.	5128.	5128.	5354.	5878.	6330	8470.	8470.	7354.	5883.	4631.	73392.	4597	4597	4597	4834.	5028.	5182.	5478	6958	8470.	7464	6237	5213.	68655.	4	70.00	5213.	5213.	0400		6419	7523	8364	7851	6463	5210.	74459.
KF BI		262.	140.	189.	267	276	, r	705	2154.	840.	478.	298.	8422.	290	244	229.	268.	222.	185.	359.	2311	955	950	723	527.	8263.		9 7	. 11.		27.8	2.40	404.	1660.	2887	1443	563	298.	9360.
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RIGHTS	LAKE	INFLOW	1192.	614.	676.	147.	149.	298.	3131.	6360.	6753	1400	1100	10 m)))	37163.	1085.	625.	619.	136.	195.	195.	2738.	6301.	5597.	7197	6882	3953.		35524.	1340.	613.	681.	165.			- 0	2208.		. 1 670	1427	7229.	4253.	40382.	
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FALLS, I	AMERIC	INFLOW	2458.	2942.	3716.	3274.	3079.	2917.	3280	424			2225	2171.		37222.1	2594.	3026.	3688	3290	3180.	3024	2659	4897	0000		. 6000	1600		36601.	2400	3300	4050	2447		2039	3 80.	6136.	7078.	6652.	2257.	2230.	1986.	AE2A7	
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O/M	· -	FLOW	1491	1514.	1965.	1665.	1578.	1414	2460		4525.	4458	2269.	1921.	1/07	26664.	1661	1502	100				1100	40 FB .	9.000		2413.			25371.	4402		. 649		1841.	1277.	1577.	4537.	6175.	7106.	2400.	1960.	1350.	4	35010.
1928-78	-	REXBRG	1057	920	1000	816.	761.	887			1/88.	1313.	244.	210.	. 226	10559.	97.8	. 61		100					747		9 6			9747	000	200	100		980	826.	795.	1271.	1667.	1493.	416.	220.	402.		11190.
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SALMON	HEISE FLOW	2320.	2539.	2585.	2088.	793.	865	1564.	11402.	24684	707	2848	47979.	2346.	1308.	1159.	1173.	1071.	848.	927.	5632.	12717.	6886.	5797.	3273.	43137.	2124.	1584.	1152.	1126.	778.	1039.	3175.	6160.	7155.	7856.	6731.	7036.	45916.
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KE RIVER DIGITAL MODE	JACK	100	219	326.	279.	364.	338.	399.	2832.	1934	660.	303.	8372.	906	308	242	247	326.	296.	412.	1561	3874	1169.	519.	402.	9565.			213						C			514.	8 199.
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	WALCO	700.	700.	700.	970.	970.	970.	9.0	951.	952.	952.		10233.	700.	700.	700.	9	700	970.	950.		951.	952	952.	688	9925.	700.	700.	780	700.	700.	970.	970.	951	951.	952.	952.	669	9944	
RIGHTS	LAKE INFLOW	2537.	1584.	152.	1126.	3074.	4280.	10411.	6460.	7699.	7168.		50704.	1861.	543.	671.	122.	92	1969.	3311.	5687.	6567.	7789.	6972.	3590.	39173.	829.	1954.	744.	160.	90.	2583.	5834.	6678.	6787.	7938	6975	4697.	45269.	
1939	TFLO	2416.	1511.	57.	993.	3009	4149.	10388.	6379.	7643.	7149.		49617.	1705.	466.	668.	30.	29.	1957.	3245.	5679.	6514	7768.	7051.	3392.	38503.	760.	1884.	716.	114.	28.	2534.	5761.	6664.	6835	7911.	6997	4520.	44724.	
INCLUDES	AMERICAN FALLS FLOW EOM OU	3515.	10247	14711.	17000.	17000.	17000.	17000.	16391.	10129.	4727.	2547.	. 136643.	3097.	5994.	9297.	12668.	15996.	17000	16497.	14141.	15968.	9702.	4064	2351.	. 126775.	4793.	6787.	9770.	12919.	15940.	17000.	17000.	14212.	10536.	4114.	9.	o.	113074.	
FALLS,	AMERIC INFLOW	2915.							6149.	1991	2219.	1950.	51259.1	2330.	3363.	3970.	3401.	3357.	2961.	2914.	3577.	8699	2105.	1877.	1860.	40414.1	3285.	3879.	3699.	3262.	3050.	3594.	5939,	4130.	3504	2035.	3283.	4635.	44296.113074	
SALMON		881.	3614	2817.	1663.	1346.	2517.	8916.	4282.	307	307.	297.	30061.	307.	1676.	2134.	1696.	1832.	1418.	923.	1693.	7093.	307.	307.	297.	19683.	1243.	2218.	2035.	1525.	1397.	1929.	3956.	2196.	1569.	307.	1513.	3036.	22925.	
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MODEL	29	183.	250.	248	246.	263.	340	854.	349.	2	54	83.	3155.	105.	224.	235.	228.	192.	291.	426.	135.	502.	87.	32.	- 4	2571.	161.	218.	222.	191	185.	303	443.	259.	166.	± €0	18.	17.	2201.	
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	SALMON F	HE I SE FLOW	2059.	1287.	931.	1185.	1072.	944	895	4745.	6107.	7273.	8520.	2366.	37385.	1570.	1265.	904	1164.	1142.	857.	1165.	7264.	12023.	6621.	5202.	3753.	42931.	2128.	2422.	1168.	1146.	1416.	1523.	2004	6759.	12417.	7893.	5467	3284.	47624.
	0/M	S OUTFLO	1724.	1041.	680.	983.	833.	642.	595.	4237.	5759.	6947.	8278	2039.	33759.	1316.	1041	656.	924.	833.	615.	595.	6489.	11676.	6356.	4947.	3396.	38844.	1845	2194	922.	922.	1211.	1311.	1770.	6161.	11703.	7469.	5045.	3008	43560.
	1928-78	ISADES EOM OL	4317.	4883.	5347.	5434.	5441.	5944.	7146.	9678.	9851.	7316.	3125.	4767.	73251.	5019.	5315.	5878.	6050.	6518.	7128.	11415.	13447.	13513.	14000.	13479.	13275.	15037.	13443	12955	13542.	13730.	14000.	14000.	14000.	13944.	13708.	11820.	10448	10838.	.156489.
	MODEL	PAI INFLOW	1861.	1608.	1144.	1070.	840.	1145.	1816.	6802.	5984.	4465.	4125.	3701.	34560.	1582.	1337.	1220.	1096.	1301.	1225.	4905.	8566.	11808.	6920.	4497.	3237.	47693.1	2040	1705	1510.	1110	1481.	1311.	1802.	6153.	11534.	5654.	3735.	3497.	41531.
	ITAL MC	TFLO	242.	246.	31.		28.	31.	30.	2152.	567.	2460.	2460.	2083.	10359.	31.	30.	31.	31.	28.	31.	29.	83.	3405.	1845	1845.	1488.	8875.	296	240	240	6	28.		30.	30.	ष	1845.	1845.	1488.	9582.
	SNAKE RIVER DIGITAL	JACKSON LAKE	4654.	4654.	4851.	5037.	5384.	5629.	6019.	6192.	8346.	6431.	4245.	2492.	63935.	2980.	3308	3678	3965	4295.	4589.	5418	8172.	8206	7697	37	5184.	63864.	2	, A &	5 18 A	FA 12	969	5869.	6180.	8354	8271.	7402.	5956.	4846.	73538.
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RIGHTS	LAKE INFLOW	1647.	328.	411	265.	134.	164.	2916.	6401.	6464	7880.	6594.	2820.	36025.	1247.	293.	187.	328.	163.	1233.	4119.	7539.	9139.	8044.	7475.	5233.	45000.	2345.	1844.	704.	140.	144.	1193.	4544.	7748.	11961.	8196.	7192.	4573.	50585.	
1939	S UTFLO	1562.	297.	331.	137.	27.	104	2901.	6321.	6497.	8028.	6515.	2526.	35247.	1096.	220.	95.	196.	27.	1218.	4241.	7421.	9250.	8171.	7389.	5160.	44484.	2312.	1828.	.909	30.	27.	1183.	4538.	7649.	11881.	8285.	7196.	4424.	49960.	
INCLUDES	ICAN FALL EOM O	701.	3763.	6542.	9334	12292.	15105.	14751.	10348.	5498.	დ	ö	ö	78335.	1346.	4334	7197.	10012.	14354.	17000.	17000.	17000.	17000.	10711.	5058.	1584.	48233.122596.	1523.	4259.	7288.	10398.	14776.	17000.	17000.	17000.	17000.	10479.	4942.	2272.	52763.123937.	
FALLS,	AMERI	2304.	3360.	3110.	2929.	2985.	2917.	2706.	2153.	1909.	2989.	6812.	2621.	36796.	2483.	3208	2958	3011	4369.	3864	4416.	7702.	9630	2508.	2207	1877.	48233.	2312	4563	3635.	3139.	4405.	3408.	4714.	7900.	12245.	2379.	2130.	1933.	52763.	
SALMON	BLKFT FLOW	307.	1641.	1419.	1350.	1553.	1358.	1046.	307.	297.	1290.	5393.	975.	16937.	668	1559	1390	1425	2262.	1746	2520.	5691.	7513.	334	307	297	25543.	404	2889	1940	1425	2585.	1934	2721.	6118.	10258.	307.	307.	297.	31185.	
1928-78 W/0	SHELLY FLOW	1481.	1621.	1414.	1414.	1610.	1414.	1304.	1977.	2208.	3366.	6937.	1632.	26379.	1294	12 B.G.	1414	1414	2138	1680	2486	7266	9276	2504	1835	1683.	34576.	1420	2987	2017	15.29	2664	1974	2875	6990	11200.	2564	1826.	1299.	39345.	
1928-	H. FORK REXBRG	822	957.	754.	791.	794.	821.	668.	1094.	768.	872.	1338.	796.	10475.	1096		780	106	1259	1071	1031	2416	1750.	870	536	854.	14306.	1001		960	832	1351	803	825	2194.	2297.	387	383	629	13068.	
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1939	ĸ	UTFLO	2258.	319.	81.	30.	27.	398.	2728.	6145.	13990.	8248.	7860.	5438.	47522.	2354.	1909.	1402.	30.	524.	3374.	3053.	6776.	10072.	8252.	5596.	4327.	47669.	3757.	3093.	1566.	3375.	1511.	3710.	7623.	12286.	8805	7920.	7309.	3991.	64946.
INCLUDES	AMERICAN FALL	EOM	310.	3468.	7148.	10881.	14080.	16705.	16808.	15828.	17000.	12995.	6888.	3292.	25403.	3712.	6816.	10009.	14043.	17000.	17000.	17000.	14591.	17000.	10352.	8045.	6712.	142280.	6329.	8934	12704.	15000.	17000.	17000.	17000.	17000.	17000.	11639.	6749.	5844.	152199.
FALLS,	AMERI	INFLOW	2608.	3477.	3761.	3763.	3227.	3023.	3000	5423.	15521.	4884	2269.	2051.	53006.	2857.	5013.	4595.	4065.	3481.	3374.	3228	4647	12866.	2235.	3767.	3217.	53344.	3476.	5698.	5336.	5671.	3511.	3710.	7798.	12586.	9203.	3195.	2912.	3304.	66402.
SALMON	BLKFT	FLOW	307.	1696.	1999.	2041.	1849.	1502.	1395.	3567.	13483.	3063	307	297.	31505.	770.	3399	2932	2422	1834	1688.	1726.	2906.	10856	307	1726.	1352.	31917.	1454.	3958.	3718.	3615.	1816.	1864.	6288.	10246.	6892.	1072.	933.	1430.	43288.
O/M	SHELLY	FLOW	1392.	1776.	2032.	2012.	1766.	1414.	1432.	4897	15119	7 162	1977.	1667.	40645.	1801.	3199	2956	2503	1848	1752.	2072.	4706.	12476	2477	2619	2017.	40425.	2188.	3781.	3770.	3674.	1761.	1712.	6000	12033.	8113.	2645.	1927.	2086.	49692.
1928-78	FORK	REXBRG	1018	1072.	1067.	1097.	970.	907.	831.	2158	3289	1736	069	803.	15540.	1247.	1367	1182	1000	1080	1214	100	1739	3136	729	1274	1075.	16497.	1313.	1416.	1221	1311.	1151.	1232.	2434.	3435.	2212.	1138.	1024.	1230.	19117.
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IVER DI	FRCSTH	•			o į	3905	3462.	3127.	2813.	2533.	1486.	ന	0	o.	17858.	0	0	o.	4501.	4315.	3981.	4393.	8	2522.	240.	ö	o.	23849.	ö	ö	0	3668.	3447.	3463	3565	3535.	2896.	1366.	; c	5		
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W-1		H. FORK ASHTON	962.	944.	784.	808	765.	861.	933.	200		1257	1058.	1017.	983.	12357.	886.	894.	756.	759.	706.	799.	1252.	2562.	1832.	1163.	1152.	1051.	13813.	1019.	897	786.	842	820.	9896	873.	1867.	2360.	12/3	1143.	978	13755.
		OUTFLO A	417.	395.	246.	304.	316.	359.	322	120	.00.	460.	461.	461.	446.	4925.	347.	370.	246.	246.	222.	246.	4 16.	921.	805.	461.	461.	446.	5188.	396.	317.	246.	246.	303.	381.	346.	703.	936	515.	461.	446.	5297.
		PARK EOM O	1157.	1157.	1284.	1350.	1350.	1350.	1350		1350.	1350.	1194.	1134.	1077.	15104.	1066.	1066.	1164.	1207.	1252.	1344.	1350.	1350.	1350.	1252.	1187.	1135.	14722.	1124.	1124.	1239.	1348.	1350.	1350.	1350.	1350.	1350.	1350.	1323.	1286.	15543.
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4	INCLUDES	'S LAKE OUTFLO I	c	Ċ	30	. n	22.	7 7			70.	40.	ö	61.	60.	357.	ö	ó	o	0	69	34.	9	40.	101	15.	61.	.09	389.	Ö	o.	o O	ö	ø.	36.	25.	54.	88	56.	61.	.09	386.
		HENRYS EOM O	777	. 4	. 670		7 0			843	843.	843.	818.	765.	717.	9792.	746.	783.	818.	838	843	843	843	843	843.	843.	773.	700.	9716.	719.	755.	788.	819.	843.	843.	843.	843.	843	843.	778.	716.	9635.
	SALMON FALLS.	HE I SE FLOW	9	. 0000	1000		120		0.44	2310.	8979.	10516.	6807.	5548	4364.	51890.	2139.	3045.	2599	2183	767.	899	4291.	11798.	21393.	8108	5077.	3762.	66062.	2760.	2853.	2694.	2376.	898	925.	1083.	6393.	16255.	11790.	5624.	4055.	57707.
	0/ ≱	S OUTFLO	000	2260				000	619	1759.	7069.	8646.	4671	3663.	3321.	40656.	1845	2736	2359	1965	555	6 15	3753.	10680	20611.	7407	4502.	3348.	60377.	2107.	2396.	2308.	1935.	555	615.	.006	5631.	14735.	11024.	4959.	3555.	50721.
	1928-78	LISADES EOM O	0000	. 200	2400	12800	12200	12963.	13717.	14000	13853.	13775.	14000	14000	14000	162707.	13847	13278	12708	10139	12768	13680.	13478	13125	13295.	14000	14000	14000.	160318.	14000.	13400.	12800.	12200.	12844.	13496.	13976.	13293.	13108.	14000.	14000.	14000.	. 161116.
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	SNAKE RIVER DIGITAL	JACKSON LAKE		5716.	2/16	5716.	6040.	6306.	6555.	6803.	8345.	8293.	7182	5635	4483.	76790.	4714	47.54	47.44	. NO. 7	50 E	5832	6401	803	B 134	. C	7065	5869.	74192.	5869	5869.	5869.	6202.	6520.	6827.	7127.	8105.	8071.	8470.	7304.	6099	82330.
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H-1	MILNER	4362. 4670.	3726.	4274	3968	2008.	ò	200.	200.	9	35498.	3017.	4709.	4205.	3250.	1343.	4033.	5944	6583.	14141.	200.	200	50.	47675.	3323.	3645.	4236.	3223.	1762.	3742.	2986.	9158.	7937	200.	200.	50.	40462.
	OUTFLO	12316. 5294. 4276.	3318.	4354	6935.	12050.	9898	11428.	10589.		89003.	5169.	5633.	4 106.	3261.	1395.	3719.	8019.	15949.	23767.	10830.	10033.	7019.	98900.	6113.	4786.	4230.	3487.	1904	4024	4376.	14644.	17882.	12082.	11184.	8164.	92876.
	WALCO	970. 970. 970.	970.	970.	.026	970.	951.	952.	952.		11314.	970.	970.	970.	970.	970.	970.	970.	970.	970.	952.	952.	699.	11332.	970.	970.	970.	970.	970.	970.	970.	910.	910.	952	952.	699.	11332.
RIGHTS	LAKE INFLOW	12979. 5294. 4276.	3318.	1647.	7222	13644.	11504.	13338.	12219.	. /06/	97302.	5729.	5633.	4 106.	3261.	1395.	3719.	8371.	17723.	25466.	12765.	11632.	7842.	107642.	6837.	4786.	4230.	3487.	1904.	4024.	4475.	15385.	19486.	14036.	12725.	9037.	100412.
1939	LS OUTFLO	12837. 4823. 3863.	2814.	1100.	.0075	14478.	12622.	14621.	13377.	/835.	99081.	5632.	5338.	3877.	2803.	941.	3487.	8297.	19162.	27125.	14582.	13096.	8875.	.113215.107642.	7016.	4614.	3819.	2910.	1616.	3724.	4011.	14635.	19752.	15139.	13847.	9859,	100942.
INCLUDES	AMERICAN FALL FLOW EOM O	11000.	15000.	17000.	200	17000	16882.	11629.	6395.	1034.		9000	11000.	13000.	15000.	17000	17000.	17000.	17000.	17000.	15127.	12076.	10744.	170948.	9000	11000	13000.	15000.	17000.	17000.	17000.	17000	17000.	14870.	87 18	5714.	98373, 162302, 100942, 100412
FALLS,	AMERI Inflow	8 169. 6823. 5863.	4814.	3100.	3,400.	14761	12912.	10004	8634.	9198.	95161.158440.	7196.	7338.	5877.	4803.	2941.	3487.	8477.	19458.	27537.	13369.	10610.	7814.	96248.118907,170948	5415	6614.	58 19.	4910.	3616.	3724.	4183.	14889.	20184.	13686.	8241.	7093.	98373.
SALMON	BLKFT FLOW	6315. 5219.	3188.	1716.	1965	13093	11021	8 195.	6908	7258	74780.	5061.	5314.	3979.	2886.	1457.	1473.	6763.	17171.	25679.	11690.	8778	5997.	96248.	2934	4692.	3967.	3076.	1762.	1675.	2307.	12119.	17445.	11297.	6743.	4577.	71593.
1928-78 W/D	SHELLY	6832. 5048.	3185.	1814	1860.	15649	14711	11468	9727	8790.	88716.	5699	5217.	4075.	3089.	1587.	1754.	7502.	19939.	29552.	14801.	11113.	8055.	.112383.	4642	4561.	4027.	3203.	1908.	1812.	2610.	12733.	20958.	15149.	8421.	6832.	86855.
1928-	H. FORK REXBRG	4126.	1396.	1270.	1299.	7501	ROS	7824.	6876.	5163.	49384.	5335.	2797.	1406.	1251.	1109.	1444.	3330.	13944	17889.	16402.	13908.	11445.	90259.	49R4	2595.	1445.	1195.	1247.	1347.	1725.	6432.	14022.	14068.	11079.	9068	69209.
MODEL	TT RIV OUTFLO	339. 354.	303.	261.	314.	797	920	900	427.	345.	6131.	1316.	632.	286.	249.	200.	367.	486.	2806.	4899.	3981.	2908	2655.	20785.	746	408	295.	233.	227.	274.	309	743.	2707.	2915.	1702.	1289.	11848.
DIGITAL M	TT RIV	850. 440.	307.	261.	315	787	1546	1054.	820.	601.	8549.	1465	705.	296.	254.	201.	368.	503	3235	5540.	4520.	3292.	2956.	23335.	96	461.	305	238.	228.	275.	320.	904.	3339.	3552.	2118.	1592.	14293.
RIVER DI	H. FORK STANTY	2548. 1667.	1090	992.	1060.	861.	. ער הרו	3550.	3042.	2711.	27597.	1648.	1309.	940.	973.	862.	1021.	1355.	4209.	5357.	3302.	2659.	2403.	26037.	2106	1325.	1045.	1043.	1068.	1141.	934	2762.	6082.	4465.	2932.	2783.	27688.
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PPER SNAKE RIVER		IVE		DIGITAL N	MODEL	1928-7	1928-78 W/D	SALMON FALLS,		INCLUDES TOTAL H	S 1939 H.FORK	RIGHT
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PICHIC		LAKE	3739.	3249.	4031.		1808.	3/12.	7184	22826.	19532.	12083.	9838	2	97654.	9930.	4045.	3595.	2835.	1468.	3923.	7042.	7350.	7720.	8859.	8234.	4168.	69169.	511.	490.	619.	166.	146.	2307.	8865.	15787.	9313.	11162.	10550.	4086.	64002.	
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	INCLUDE	AMERICAN FALL FLOW EOM O	6694.	10436.	13000.	15000.	17000.	17000.	17000.	17000.	17000.	12833.	11629.	13060.	167652.	9000	11000.	13000.	15000.	17000.	17000.	12839.	8189.	4232	1543.	154.	0	. 108958.	3439.	6948	10255.	13487.	16207.	17000.	17000.	17000.	16244.	10724.	4900.	5544.	72087.138747.	
	FALLS,	AMERI Inflow	4801.	6865.	6396.	5187.	3651.	3608.	7142	24176.	20242.	8624.	9095	7369.	107155.	5649.	5828.	5377.	4723.	3347.	3946.	3989.	2702.	4491	7221	7399	4258	58931.	3550.	3821.	3838.	3262.	2747.	3112.	9171.	16011.	9030	6818.	5737.	4990.	72087	
	SALMON	BLKFT FLOW	2210.	4797.	4252.	3162.	1686.	1546.	5182.	21952.	18089.	6691.	6907	5316.	81789.	2442.	3504	3361	2751	1650.	2160	2348	307	1386	4202	4475	1190.	29777.	686	1977	1961.	1542.	1217.	1371.	7174.	13596.	6819.	4431.	3128.	2753.	46958	
	1928-78 W/O	SHELLY	3822.	4629.	4490.	3553.	1901.	1948.	5077.	23046.	19886.	10370.	9203.	7006.	94930.	3308.	3233.	35 16	2965	1731.	2251	3801	2716	SAGS	5257	0000	1956.	39484.	1943	2083	2095	1679	1356.	1577.	7460.	15514.	9948.	7770.	5477.	4350.	61252	
1	1928-	H. FORK Rexbrg	4537.	2072.	1354.	1264.	1170.	1219.	1974.	10155.	13509.	12289.	9477.	7866	66887.	3653.	1573.	1041	876	921	1637	3766	AARO	- FEE	0 00 00 00 00 00 00 00 00 00 00 00 00 0	. FCE'S	2417.	37572.	1044	708	900	722	290	785.	2209.	7085.	5571.	6267.	5416.	2954.	34395	0 0 0
	MODEL	TT RIV	1668.	486.	355.	259.	235.	258.	584.	1862.	4620.	4550.	2803.	2461.	20141.	640.	277	263	247	219	100	1344	7.72	1410.	4734		670.	9100.	r G	242	217	107	184	339	1459.	5042.	5672.	6270.	5351.	2784.	28273	404
	DIGITAL M	TT RIV	1841.	541	369.	265.	236.	259.	600.	2168.	4927.	4982.	3125.	2636.	21949.	649	311	OBC		220		1384		1824	1010	, m	873.	10448.	100	27.6	23.6	. 400		340	1475	5265	6234	6766	5711	2991.	30311	- 200
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Table 29.--Column Description, North Side Pumping Division Operation Study

Column	Description
IRRIGATION DEMAND	The irrigation demand is the amount of water, exclusive of precipitation, required for crop production, including losses in application, distribution, and operation of the system. This value represents the combined irrigation demands of the existing Unit A lands (82 percent) and the proposed new Unit A lands (18 percent).
IRRIGATION SHORTAGE	This irrigation shortage equals the irrigation demand minus the water supply (combination of natural flow and storage). The irrigation shortages experienced by the district are voluntary in nature, noting all canal companies in the upper Snake River were assigned voluntary shortages in 1934, 1935, and 1977. These shortages were implemented in an attempt to maximize water use and carryover storage; however, for the A&B Irrigation District, they had sufficient storage in 1934, 1935, and 1977, and had the voluntary shortages not been implemented, they would not have experienced irrigation shortages during the 51-year period examined.
WATER SUPPLY	The order of water use for the operation study was (1) natural flow, (2) American Falls storage, and (3) Palisades storage.
Natural flow	The portion of the irrigation supply delivered to the project lands (existing plus new) under the district's 1939 natural flow right.
Storage	The portion of the irrigation supply delivered to the project lands (existing plus new) under the district's storage rights.
A&B STORAGE (EOM)	The end-of-month content (EOM) in American Falls and in Palisades Reservoirs

YEAR	MENTH		ATION		ATION TAGE	+	WATER	SUPPL	. Y	+ -+	ASB STURA	GE (EOM)
		DEM	IAND	31:UK	1465	÷	NATURAL FLOW	STOP			AMERICAN FALLS RES	PALISADI RESERVO
		ACRE	FEET	ACRE	FEET	+	AC. FT.	AC.	FT.	+	ACRE FEET	ACRE FE
1927	CCT		2230		0		2230		0		12675	8943
1927	NOV		0		0		C		0		12676	8943
1927	CEC		O		0		O		C		12576	8943
1928	JAN		0		0	10	0		0		12676	8943
1928	FEB		C		0		0		0		12576	8943
1928	MAR		0		0		0		O		12576	8943
1928	APR		1110		- 0		1110		0		12676	8943 8943
1928	MAY	1	LOCTO		0		10070		0		12676	8943
1928	NUL		13630		0		13630	-	0		12676	3208
1928	JUL		18620		0		12707		913		40913	3208
1928	AUG	1	15400		0		0		400		25513	5177
1928	SEP		7600		0		C 		600 		17913	
1928	TOTAL	i	58660		0		39747	28	913			
1928	CCT		2050		0		2050		0		14503	4191
1928	VON		0		0		C		0		14503	4191
1928	DEC		0		0		0		0		14503	4191
1929	NAL		. 0		0		0		0		14503	4191
1929	FEB		0		C		C		0		14503	4191
1929	MAR		0		0		0		0		14503	419
1929	APR		650		0		650		0		14503	419: 419:
1929	MAY		9750		0		9750		0		14503	320
1929	JUN		13300		0		13300		0		46826	320
1929	JUL		18620		0		3169		451		31375	320
1929	AUG		15610	**	0		0		510		15765	464
1929	SEP		708C		0		2050) 	030		10734	
1929	TOTAL		67050		0		30969	36	091	5 4D 4		
1929	CCT		1900		0		1900		O		10100	437
1929	NOV		0		0		0		0		10100	437
1923	DEC		0		0		0		0		10100	437
1930	JAN	ā	0		0		C		0		10100	437
1930	FEB		0		0		0		0		10100	437
1930	MAR		0		0		0		0		10100	437
1930	APR		2430		0		2430		0		10100	437
1930			7850		0		7850	72	0		46826	320
1930	JUN		12120		٥		4226		894		38932	320
1930	JUL		13950		0		0		950		19982	320
1930		8	13550		0		. 0		550		6432	320 870
1930	SEP		6580				0	ó 	580		0	0 I V
	TETAL		63380		0		16406	1. 4	974			

UPPER SNAKE RIVER DIGITAL MODEL 1928-78 MINIDOKA NORTH SIDE EXTENSION

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		DEM	CNA	2404	TAGE	+	NATURAL	STOR	RAGE	+	AMERICAN	PALISADE.
						+	FLOW		TER		FALLS RES	RESERVOI
		ACRE	FEET	ACRE	FEET	÷	AC. FT.	AC.	FT.	+	ACRE FEET	ACRE FEE
1933	CCT		2180		0	-	2180		0	-	4784	89372
1933	VON		0		0		0		0		4784	89372
1933	DEC		0		0		0		0		4784	89372
1934	JAN		0		0		0		Ð		4784	89372
1934	FEB		0		0		0		0		4784	89372
1934	MAR		0		0		0		0		4784	89372
1934	APR		2340		0		2340	_	0		42131	32089
1934	MAY		8529		-2558		C		971		36160	32089
1934	JUN	1	10139		-3041		0		098		29062	32089 32089
1934	JUL		14799		-4439		C		360		18702	32039
1934	AUG	1	13479		-4043		C		436		9266	70477
1934	SEP		6270		0		0	6	270 		2996 	
1934	TOTAL		57736		14081		4520	39	135			
1934	BCT		2230		0		2230		٥		2994	70419
1934	NOV		O		0		0	* *	0		2994	70419
1934	DEC		0		0		٥		0		2994	70419
1935	JAN		0		0		0		0		2994	70419
1935	FEB		0		0		0		0		2994	70419
1935	MAR		G		0		0		0		2994	70419
1935	APR		650		D		65C		0		2994	70419
1935	MAY		8500		C		9500		. 0		2994	70419
1935	JUN		11739		-1126		10613		0		46526	32089
1935	JUL		17059		-9382		0		677		39149	32089
1935	AUG		14810		-2145		0		665		32485	32089
1935	SEP		8020		0		0	8 	020		24465	71395
1935	TOTAL		63008		18653		21993	22	362	p cyp-480		
1935	DCT		2040		0		2040		0		19879	58014
1935			0		0		C		0		19379	5801
1935			C		0		. 0		0		19879	5301
1936			0		0		0		0		19879	5801
1936			0		0		0		0		19379	5801
1936			0		0		C		0		19879	5801
1936			930		0		980		0		19879	5801
1936			10650		0		10650		0		19379	5801
1935			12830		0		12830	22	0		46326	3208
1936			13540		0		C		540		28286	3203
1936			15450		0		0		450		12336	3208
1936	SEP		7170		0		C	7	170		5666	8299
1936	TCTAL		67660		0		2650C	41	160			
											Λ&R 71	 5

	5)												
•	YEA	R	MENTH		ATION		ATION	+	WATER	SUPPLY	÷	AEB STORA	GE (EDM)
				DEM	IAND	SHOR	TAGE	40	NATURAL	STOPAGE	+	AMERICAN	PALISADE
				ű.				+	FLOW	WATER		FALLS RES	RESERVOI
				ACD5	FEET	ACRE	EFET					ACRE FEET	ACRE FEE
				A	PEL I	A C N L	, , , , 1 						
	193	36	CCT		2190		0		2190	0		5823	86230
	193	36	NOV		0		0		0	0		5823	86230
	193	36	DEC		0		0		0	0		5823	86230 86230
	193		MAL		C		0		0	0		5823	86230
	193		FEB		0		0		0	0		5823	85230
	193	37	MAR		C		0		0	0		5823	86230
	193	37	APR		1030		0		1030	0		5823	86230
	193		MAY		9750		0		9750	0		5823	32089
	193	37	NUL		11800		0		6355	5445		41381	32099
	193	37	JUL		18380		0		0	18380		23001	32089
	19:	37	AUG		15250		0		0	15250		7751	
	193	37	SEP	W 505	7300		0		0	7300		451	83691
	19	 37	TETAL	(65700		0		19325	46375			
•	19:	 37	OCT		1990		0		1990	0		447	82834
	19.		NOV		0		0		0	0		647	82884
	19:		DEC		0		Q		0	0		447	82884
	19:		JAN		Ö		0		0	0		447	82884
	19		FEB		Ö		Õ		Ō	0		447	82884
)	19		MAR		Ö		0		0	. 0		447	82884
	19.		APR		990		٥		990	0		447	82884
			MAY		9520		ő		9520	o o		447	82334
;	19:				12290		o o		12290	C		447	82884
	19		אטר		16780		G		5828	10952		35374	32089
	19		JUL				n	62	0	15180		20694	32039
d	19 19		SEP	vo.	15180 8440		ŏ		Ö	8440		12254	89771
			TOTAL		6519C		0	-	30618	34572			
;					1000		0		1800	0		12212	89463
	19.		DCT		1800		0		1800	o o		12212	89463
)	19		NOV		Ü	2	0		Č	ō		12212	89463
•	19		DEC		Ü		0		0	Ö		12212	89453
	19		JAN		0		0		0	0		12212	89463
)	19		FEB		0		0		0	0		12212	89463
ž	19		MAR		0		U		0	30 - 5		12212	89463
	19		APR		2270		Ü		2270	0		46825	32089
¥		39	MAY		10600		0		10600	12370			32089
j		33	JUN		12870		0		0	12870		33956 15776	32089
-		39	JUL		18180		0		0	18180		15776	32089
	19	39	AUG		15570		0		C	15570		205	78773
	19	39	SEP		7620		0			7620			10110
3	19	39	TCTAL		68910		0		14670	54240			
2								V166.6				A&B 716	6

	j6:									22072		
YEAR	MENTH		ATION		ATION	+	WATER	SUPPL	Υ	+	AEB STORA	GE (EDM)
		DEM	CNA	SHOR	TAGE	+ -	NATURAL	STOR	PAGE	+	AMERICAN	PALISADE
						+	FLOW		ER		FALLS RES	RESERVOI
	a ao o a a	ACRE	FEET	ACRE	FEET	+	AC. FT.	AC.	FT.	+	ACRE FEET	ACRE FEE
											161	90800
1939			1390		0		1390		0		161	90800
1939			0		0		0		٥		161	90800
1939			0		0		0		n		161	90800
1940			0		0		0	8	D		161	90800
1940			0		0		0		0		161	90800
1940					0		1270		a		161	90800
1940			1270		0		3169	6	601		40225	32089
1940		4	9770		0		2200		250		26975	32089
1940			13250 18620		0		0		620		8355	32089
1940			15530		n		Ö		530		0	23963
1940		2	6370		ō		5815		555		0	76573
			56200		0		11644		556			
1941	TOTAL											
1940	DCT		1190		0		1190		0		8 9	8273
1940			0		0	4)	0		0		89	8273
1940	DEC		0		0		O		0		89	8273
194	L JAN		0		0		0		0		89	8273 8273
194:	L FEB		0		0		C		0		89	
194	MAR		0		0		0		0		89	8273 8273
194.	L APR		1820		0		1820	_	0		89	8273
194	1 MAY		9890		0		0		890		89 25485	3208
194	1 JUN		11450		0		C		450		7816	3203
194	1 JUL		17670		0		0		670		, 0	2382
194		8	15110		0		0		110		. 0	7492
194	1 SEP		6960		0 		3178		102			
194	1 TOTAL		64090		0	rep eds	6188	57 	902			
194	1 CCT		1460		0		1460		0		. 0	6985
194	1 NOV		0		. 0		0		0		0	6985
194			0		0		0		0		. 0	6985 6985
194	2 JAN		0		0		0		. 0		Ü	6985
194	2 FEB		0		0		0		0		0	6985
194	2 MAR		C		0		0		0		2	
194	2 APR		1490		0		1490	**	Q		0	6985 6985
194	2 MAY		8950		0		8950		J		2/705	
194	Z JUN		11630		0		1589		041		36785	3208 3208
194			17930		0		0		930		18355	3205
194	Z AUG		15280		0		C		280		3575	
194			7550		0.		C		7550		O 	8463
194	2 TOTAL		64290		0		13439	50	801			
										- 3	A&B 717	7

YEAR	MENTH	IRRIG				+	WATER	SUPPLY		+ +	465	STOR	AGE (EDM)
		DEM	AND	SHOR	TAGE	* =	NATURAL	STORA	GF	+ Δ	MER:	ICAN	PALISAD
						+	FLOW	WATE				SRES	RESERVO
							AC. FT.	Ar E	 T		CPF	FFFT	ACRE FE
		ACRE	FEET	ACRE	ree!	*	ALe Fie	ALe F	14 				
1942	ECT	B	1720		0		1720		0			0	8148
1942	NOA		0		0		0		0			0	8148 8148
1942	DEC		Õ		0		0		n.			0	8148
1943	JAN		0		0		0		n			o	3148
1943	FEB		0		0		0		0			Ö	8143
1943	MAR		1760		0		1760		0			Õ	8148
1943	APR		0240		ŏ	92	10240		0			ō	8148
1943 1943	JUN		0910		ő		10910		0			0	8148
1943	JUL		8080		o		10589	749	1		3	9335	3209
1943	AUG		5620		0		0	1562				3715	3208
1943		•	7890		ō		2113	577			1	7933	8987
1943	TOTAL	6	6220		0		37332	2888	8				
1943	ECT		1810		0		1810		0		1	7990	9028
1943			0		0		0		0			7990	9028
1943			O		0		. 0		0			7990	9028
1944		88	0		0		. 0		0			7990	9028
1944			0		0		0		0			7990	9029
1944	MAR		0		0		0		0			7990	9028
1944	APR		970		0		970	N V	0			7990	9028
1944	MAY		8150		0		7404	74				7990	9028
1944	JUN		9770		0		0	977				7990	9028
1944	JUL	2.2	7910		. 0		0	1791				8400	3208
1944	AUG	1	15370	54	0		0	1537				3030	3209 8970
1944	SEP		8130		0		<i>5</i> 799	233				699 	
1944	TCTAL	6	2110		0		15983	4612	27				
1944	CCT		1870		0		1870		0			632	8110
1944	NOV		C		0		0		0			532	8110
1944	DEC		0		0		0		0			632	8110
1945			0		0		0		0			632	811
1945			0		0		0		0			632	8110 8110
1945			0		0		0		0			632	\$110
1945			950		0		950		0			532 532	8113
1945			9420		0		9420		0			632	811
1945			1500		0		11500	1 2 2 2	0		7	4574	320
1945			08081		0		5828 0	1225 1534				9234	320
1945		2	7910		0		0	791				1324	907
	TCTAL		55070		0		29568	3550) 2				
											Α.	&B 7	18
		(5)										ו שא	10

UPPER SNAKE RIVER DIGITAL MCDEL 1928-78 MINIDOKA NORTH SIDE EXTENSION .

YEAR	MONTH	IRRIGAT DEMAN		IRRIG SHOR	ATION	+	WATER	SUPPLY	+	AEB STORA	GE (EDM)
		DEMAN	U	31,31		*	NATURAL			AMERICAN	PALISADE
						+	FLOW	WATER	+ 	FALLS RES	RESERVOI
		ACRE F	EET	ACRE	FEET	+	AC. FT.	AC. FT.	+	ACRE FEET	ACRE FEE
1945	OCT	13	90		0		1390	0		10515	84296
1945	NON		0		0		O	0		10515 10515	84296 84296
1945	DEC		0		0		0	0		10515	84296
1946	JAN		0		0		0	0		10515	84296
1946	FEB		0		Ü		0	G		10515	84296
1946	MAR		0		0		1390	0		10515	84296
1946	APR	13			0		10310	0		10515	84296
1946	MAY	103			٥		12740	ລ		10515	84296
1946	אחר	127			0		1592	16438		30388	32089
1946	JUL	180 153			٥		0	15310		15078	32089
1945 1946	AUG Sep		40		0		4766	1974		13105	90696
1946	TGTAL	659	10	· ·	0		32188	33722		********************************	
1946	GCT	13	60		0		1360	0		12120	83880
1946	VON		0		0		C	0		12120	83880
1946	DEC		0		0		0	0		12120	83880
1947	JAN		0		0		0	0		12120	83880
1947	FEB		0		0		C	0		12120	83880
1947	MAR		0		0		C	0		12120	83880
1947	APR	16	50		0		1650	0		12120	83880
1947	MAY	108	00		0		10800	0		12120	83830
1947	JUN	112	30		. 0		11230	0		12120	83830
1947	JUL	186	50	e8 =	0		1067	17583		29243	32089
1947	AUG	155		7	0		C	15550	2.	13693	32039
1947	SEP	71	30		0		6133	997		12596	90506
1947	TCTAL	663	70		0		32240	34130			
1947	CCT	14	40		٥		1440	0		11327	80749
1947			0		0		C	0		11327	80749
1947			0		0		. 0	0		11327	80749
1948			0		0		0	0		11327	80749
1948			0		0		0	0		11327	80749
1948			0		0		0	0		11327	80749
1948		15	30		C		1530	C		11327	80749
1948		103			O		10320	0		11327	80749
7240		128			G		12810	0		11327	80749
1948		182	200		0		3710	14490		32336	32089
	JUL				•		0	15730		15505	3208
1948		157			0		10				
1948 1943	AUG	157	730 510		0 0		6355	1255		15352	9053
1948 1943 1948 1948	AUG	157	510 	2			10				

YEAR	R MC	NTH		ATION		ATION	+	WATER	SUPPLY	+	AES STORAC	GE (EDM)
			DEM	DNA	SHUK	TAGE	++	NATURAL FLOW	STORAGE WATER		AMERICAN FALLS RES	PALISADE RESERVOI
	ده جه و		ACRE	FEET	ACRE	FEET	·		AC. FT.	+	ACRE FEET	ACRE FEE
					,	0		1170	0	-	13045	76931
194		T3C	8.*	1170		0		0	ő		13045	76931
194		IDV DEC		0		0		Õ	0		13045	76931
194 194	(E) 191	JAN		0		0		Ō	Ö		13045	76931
194		EB		Č		ō		Ď	Ō		13045	76931
194		'AR		D	*	0		0	D		13045	76931
194		APR		1550		Ö		1550	0		13045	75931
194		MAY	1	0760		Ö		10760	0		13045	76931
194		אטנ		2700		ō		12700	0		46826	32089
194		JUL		8570		٥		0	18570		28256	32089
194		AUG		5350		0		0	15350		12906	32089
194		SEP		7310		0		0	7310		5596	90421
194	9 T	GTAL		57410		0		26180	41230			
194	9	DCT	~~ ~~ ~ *	1390		0	dia dia	1390	0		5302	85672
194		NOV		0		0		0	0		5302	85672
194		DEC		Ō		0		0	0		5302	85672
195		JAN		0		0		0	0		5302	85672
195		FEB		0		0		0	0		5302	85672
195		MAR		0		0		0	D		5302	85672
195		APR		1220		0		1220	0		5302	85672
195		MAY		9930		0		9930	0		5302	85672
195	0.000	NUL		13250		0		13250	0		5302	85672
195		JUL	7. 9	18770		0		9522	9248		37578	32089
195		AUG	S 8	15470		0		C	15470		22108	32039
195		SEP		8320		0		0	8320		13788	90796
195	0 T	CTAL		68350		0		35312	33038			
195	0	CCT		1680	۲.	0		1680	0		13776	90719
195	0	NOV		0		0		0	٥		13776	90719
195		DEC		0		0		0	0	v	13776	90719
195	1	JAN -		0		0		C	0		13776	90719
195	1	FEB		0		C		0	0		13776	90719
195		MAR		O		0		0	0		13775	90719
195		APR		1540		0		1540	0		13776	90719
195		MAY		9650		0		9650	Ō		13776	90719
195		NUL	130	13200		0		13200	0		13776	90719
195		JUL		18740		0		5286	13454		33372	32089
195	1	AUG		14870		C		. 0	14870		18502	32039
195	1	SEP		8490		0		0	8490		10012	90444
195	51 T	CTAL		68170		0		31356	36814			
											A&B 720)

YEA	R 1	MONTH		ATION	IRRIG SHDR	ATION	+	WATER	SUPP	LY	+	ARB STORA	(MCB) BE
			DEM	CNAI	2508	IAGE	+	NATURAL FLOW	- 15-00 SYLVED	RAGE TER		AMERICAN FALLS RES	PALISADE RESERVOI
			ACRE	FEET	ACRE	FEET	+	AC. FT.	. AC.	FT.	+	ACRE FEET	ACRE FEE
				2020		0		2020		0		9300	84011
195		DCT		2020		0		0		0		9300	84011
195		NOV		0		. 0		C		D		9300	84011
195		DEC		0		0		. 0		0		9300	84011
195		JAN Feb		n.		Ô		C	20	0		9300	84011
195		MAR		Ô		n		ō		0		9300	84011
195		APR		1050		0		1050		0		9300	84011
195 195		MAY	1	10960		ō		10960		0		9300	34011
195		JUN		13940		Ö		13940		0		9300	84011
195		JUL		19190	20	0		5286	13	904		32922	32089
195		AUG		6210		Ō		0		210		16712	32089
195		SEP	•	8730		0		0	8	730	-	7982	90464
195	2	TCTAL	,	72100		0		33256	38	844			
195	2	DCT		2290		0		2290		0		7415	84033
195		NOV	29.7	0		0		0		O		7415	84033
195		DEC		0		0		0		0		7415	84033
195		MAL		0		0		0		0		7415	84033
195		FEB		0		0		O		0		7415	84033
195		MAR		0		0		0	8	0		7415	84033
195	3	APR		1870		0		1870		0		7415	84033
195	3	MAY	84 8	10430		0		7404		1025		7415	84033
199	3	JUN		12130		0		12130		0		7415	32089
195	3	JUL		19230		0		0		230		24570	32089
195	3	AUG		15840		0		. 0		340		8730	32089
195	3	SEP		8450	***	0		0		450		280	90030
195	3	TOTAL		70240		. 0		23694	4(546			
195	3	CCT		1960		0		1960		0		255	81783
199	53	NOV		. 0		C		0		ט		255	81783
195	53	DEC		0		0		0		0		255	81783
195	54	MAL		0		0		0		0		255	81733
195	54	FEB		0		0		. 0		-0		255	81783
19	54	MAR		0		0		C		0		255	81783
195		APR		1940		0		1940		0		255	81.783
19	54	MAY		11130		0		11130		0		255	81783
195	54	JUN		12320		0		12320		0		255	81783
19	54	JUL		13540		0		4751		3779		33047	32089 32089
19:	54	AUG		15610		0		C		5610		17437	
19	54	SEP		8280		0		0) 	8280		9157	89641
19	54	TETAL		69730		0		32111	3	7669			
							xe na T ilila			· · · · · · · · · · · · · · · · · · ·		A&B 72	1

UPPER SNAKE RIVER DIGITAL MCDEL 1928-78 MINIDDKA NORTH SIDE EXTENSION

-												
YEAR M	4CNTH	IRRIG	ATION		ATION	+	WATER	SUPPL	Y Y	+	AEB STORAG	E (EDM)
		DEM	AND	SHOR	TAGE	* -	NATUD:	 crno	ecs	- et- a	AMERICAN	PALISAD
						+	NATURAL FLOW	WAT			FALLS RES	RESERVO
									a-c=-			
		ACRE	FEET	ACRE	FEET	÷	AC. FT.	AC.	FT.	+	ACRE FEET	ACRE FE
954	DCT		1640	8	0		1640		0		8544	8363 8363
954	NOV .		C		0		0		0		8544 8544	8363
1954	DEC		0 .		0		0		0		8544	8.363
1955	JAN		0		0		0		0		8544	8363
1955	FEB		0		0		0		0		8544	8363
1955	MAR		0	53	0		720		٥		8544	8363
1955	APR		720		0		9550		n		8544	8363
1955	MAY	3	9550		0			1.7	206		45620	3208
1955	JUN		13360		0		12154	185			27040	3208
1955	JUL		18580		0		0	159			11130	3208
1955	AUG		15910		٥		1589		111		4119	8989
1955 	SEP		8600									
1955	TCTAL		68360		0		25653	427	707 			
1955	CCT	0	1490		0		1490		C		3799	8199
1955	NOV		0		0		0		0		. 3799	8198 8198
1955	DEC		0		0		0		0		3799 3799	819
1956	MAL	6)	0		O		0		0		3799	819
1956	FEB		C		0		0		0		3799	8198
1956	MAR		0		. 0				0		3799	819
1956	APR		1870		0		1870 10570		n		3799	819
1956	MAY		10570		0		13010		n		46826	819
1956	JUN		13010		0		13010	19	020		27306	320
1956	JUL		19020		0		C		020		12786	320
1956 1956	AUG SEP		15020 7820		0		1064		756		6030	899
	TGTAL		68800		0		28004	40	796	. — —		
					0		1460		0		5359	798
1956			1460		o		0		٥		5359	798
1956	VOV	500	0		n		o		0		5359	798
1956	DEC		0	2.	ō		0		0		5359	798
1957	JAN FEB		C		O		C		0		5359	798
1957	MAR		. 0		C		. 0		0		5359	798
1957	APR		1100		Ō		1100		0		5359	798
1957	MAY		9820		0.		9820		0		5357	798
1957	JUN		12930		0		12930		0		5359	798
1957	JUL		19070		0		4236	14	334		31992	320
1957 1957	AUG		15360		0		C		360		16632	320
1957	SEP		7970		0		0	7	970		8562	902
1957	TOTAL		67710		0		29546	38	164	. 0		351
								aa			A&B 72	2
		•.										

	YEAR	MENTH		ATION		ATION	+	TAW	ER	SUPPL	Y	÷	488	STOR	RAGE	ED	M)
			DEM	DNA	SHOR	IAGE	ф=	NATUR	221	STOR	AGE	4	AMERI	CAN	PAL	.IS	ADI
							+	FLOW			ER		FALL:			ER	VD:
			ACRE	FEET	ACRE	FEET	+	AC. F	7.	AC.	FT.	+	ACRE	FEE	r ACI	E	FE
-				1390		0		139			0	-		8114	and the second second second second	84	53
	1957 1957	NOV		1390		0			0		0			8114		84	
	1957	DEC		o		Ö			0		0			8114	.00	84	53
	1958	JAN	12	Ô		Ö			0		C			8114		8 4	
	1958	FEB		Ö		0			0		0			8114		84	
	1958	MAR		Ö		0			0		0			8114		34	
	1958	APR		1530		0		153	30		0			8114		84	
	1958	MAY		10650		0		1065			0			8114		84	
	1958	JUN	5	12940		0		63:	55		585			0241		32	
	1958	JUL	1	19040		0			0		040			1201		32	
	1958	AUG		15180		0			0		180			6021		32	
	1958	SEP		7280		0		15	89 	. 5 	691 			330		8 3	94
	1958	TOTAL		68010		.0		215	14	46	496						0 em 6
	1958	CCT		2010		0		20			.0			317		8 5	
	1958	NOV		0		0			0		0			317		8 5	
s ,	1958	DEC		0		0			0		0			317		8 5	
	1959	JAN		0		0		34	0		0			317		85	
	1959	FEB		0		0			C		0			317		8 5	
	1959	MAR		0		0			0		0		,	317 6826		8 3 3	
	1959			2130		0		21			0			6826		32	
	1959			9450		0		94			0			6826		32	
	1959			12500		. 0		125 42		1.4	534			2292		3	
	1959			18770		0		44	0		020			7272		3	
	1959			15020 6470		0		31			292			3973		8	
		TOTAL		66350		0		335	04	32	846					(4) edia 41	
	1959	GCT		970		0		<u>-</u>	70		0		1	1348		7	49
	1959			0		0			0		0		1	1848	1	7	49
	1959			ō		0			0		0		1	1848	B	7	
	1960			0		0			0		0			1848		7	
	1960			0		0			0		0			1348			49
	1960			٥		0			C		0			1848			49
	1960			1710		. 0		17	10		٥			6826			20
	1960			10590		٥			0		590			6236			20
	1960			13230		0			0		230		2	3006			20
	1960			19230		0			0		230			3776			20
	1960			15150		0			O		150			0			92
	1960			8070		0			0	8	070					5 	14
	1960	TCTAL		68950		0		26	80	56	270						
													Ā	&B 7	723	v sami sl ik	residente.
			S•														

YEAR	MONTH	IRRIG DEM		IRRIGA		+	WATER	SUPPLY	+	A&B STORA	GE (EDM)
		UEM	UNA	24541	AJE	*	NATURAL	STORAGE	*	AMERICAN	PALISADE
						+	FLOW	HATER	+	FALLS RES	RESERVOI
		ACRE	FEET	ACRE	FEET	+	AC. FT.	AC. FT.	+	ACRE FEET	ACRE FEE
1960	SCT		1800		0		1800	٥		0	41332
1960	NOV		0		0		. 0	0		0	41332
1960	DEC		0		0		Ō	0		0	41332
1961	JAN		0		0		C	0		0	41332 41332
1961	FEB		0		0		0	0		0	41332
1961	MAR		0		0		0	0		45687	32089
1961	APR		1940		. 0		1940	0010		35777	32089
1961	MAY	_	9910		0		0	9910 12500		23277	32089
1961	JUN		2500		0		0	19250		4027	32089
1961	JUL		9250		0		0	14420		4021	20319
1961	AUG	1	4420		0		5590	0		o o	6331
1961	SEP		5590								
1961	TCTAL	6	5410		0		9330	56080			
1961	DCT		1500		0		1500	. 0		632	15479
1961	VOV		0		0		0	0		632	16479
1961	DEC		0		0		С	0		632	16479
1962	JAN		0		0		0	0		632	16479
1962	FEB		0		0		C	0		632	16479
1962	MAR		0		0		0	0		632	16479
1962	APR		1770		0		1770	Ü		632	16479
1962	MAY		9990		0		9990	0		632 46826	16479 32089
1962	JUN		2600		0		12600	15046		30880	32089
1962	JUL		9640		0		3694	15946		15160	32039
1962	AUG		9140		0		. 0	15720 9140		6020	89921
1962	SEP										
1962	TOTAL		70360		0 		29554 	40806			
1962	GCT		1990		0		1990	Ò		5443	81309
1962			0		0		0	. 0		5443	81309
1962			0		0		0	0		5443	81309
1963			0		0		0	0		5443	81309
1963			0		0		0	0		5443	81309
1963			0		0		0	0		5443	81309
1963			1380		0		1380	0		5443	81309
1963			9830		0		9830	0		5443	81309
1963			12270		0		12270	0		46826	81309
1963		22	20100		0		1067	19033		27793	32089
1963		"	15510		0		0	15510		12283	32039
1963	SEP		3180		0		1589	6591		5692	90429
	TOTAL		59250		0		23126	41134		200 ²⁷	
1963	10172		.,		_						

YEAR	MCNTH	IRRIGA		IRRIG SHJR		4	WATER	SUPP	LY	+ -+-	AEB STORA	GE (EDM)
		DEMA	נאט	2HJK	LAGE	+	NATURAL	STC	RAGE	+	AMERICAN	PALISADE.
						+	FLCW		TER	+	FALLS RES	RESERVOI
		ACRE	FEET	ACRE	FEET	+	AC. FT.	AC-	FT.	+	ACRE FEET	ACRE FEE
1963	CCT	2	2700		0		2700		0		5078	80679
1963			0		0		0		0		5078	80679
1963			0		0		0	-	0		5078	80679 80679
1964	JAN		0		0		0		0		5078	80679
1964	FEB		0		. 0		0		0		5078 5078	80679
1964	MAR		0		0		0		0		5078	80679
1964	APR		720		0		720		0		507B	80679
1964	MAY		0800		0		10080	8	0		5078	80679
1964	אטג		2190		0		12190	• •			34582	32089
1964			0190		0		7946		244		18142	32089
1964			6440		0		0		990		9152	89773
1964	SEP		8990 									
1964	TOTAL	7	1310	,	0		33636	37	674			
1964	DCT		2620		0		2620		0		8283	81253
1964			0		0		C		0		8283	61253
1964	. DEC		0		0		0		0		8283	81253 81253
1965	MAL 6		0		0		Ü		0		8283	81253
1969	5 FEB		0		0		0		0		8283 8283	81253
196			0		0		0		0		8283	81253
196			1770		0		1770		0		8283	81253
196			0880		0		19680 13370		0		8283	81253
196			3370		C		5303	1 7	4357		32469	32089
196			9660		٥		0		5450		17019	32089
196. 196		111000	5450 8060	2/A 84 ₁₁	٥		Ö		8060		8959	90425
									 7867			
196	5 TCTAL	7 	1610	<u></u>	0		33743					
196	5 CCT		3010		0		3010		0		8656	87366
196			0		0		0		0		8656	87366
196			0		0.		0		0		8656	87366
196			0		. 0		0		0		8656	87366
196	6 FEE		0		0		. 0		C		8656	87366 32039
196	6 MAR		0		0		. 0		0		46826 46826	32089
196			2530		C		2530		0		35586	32089
196			1240	(8)	0		0		1240 2500	2	23086	32089
196			.2500		0		. 0		2300 9080		400 s	32039
196			9080		0		0		5460		4005	19117
196		1	5460		Ü		0		8320		n	57100
196	6 SEP		8320		U 							
196	6 TOTAL	7	2140		0		5540	5	6600			
								SOC S	88		A&B 72	5

YEAR M	GNTH	IRRIG			ATION	4	WATER	SUPPLY		3A +	B STOR	AGE (EJM)
		CEM	ANU	SHUK	TAGE	+	NATURAL	STORA	GE	+ AME	RICAN	PALISADE
						+	FLOW	WATE			LS RES	RESERVOI
	page villeallir ugp aftir d	ACRE	FEET	ACRE	FEET	+	AC. FT.	. AC. F	Τ.	+ ACR	E FEET	ACRE FEE
1966	GCT		2670		0		0	267			0	54029
1966	NOV		0		0		O		0		0	54029
1966	CEC		0		0		0		0		0	54029
1967	JAN		0		0		0		0		0	54029 54029
1967	FEB		0		0		0		0		0	
1967	MAR		C		0		C		0		Ü	54029
1967	APR		1820		0	63	1820		0		0	54029
1967	MAY	1	0270		0		10270		0		U	54029
1967	NUL		2560		0		12560	_ , _	0.		22.24	54029
1967	JUL	2	0030		0		5828	1421			32624	32089
1967	AUG	1	6540		0		0	165			16084	32089
1967	SEP		9430		0		0	94:	30 		6654	88655
1967	TETAL	7	3320		0		30478	428	42			
1967	CCT		2540		0		2540		0		6354	84658
1967	YCM		0		0		C		0		6354	84658
1967	DEC		0		0		- 0		0		6354	84658
1968	JAN		0		0		0		0		6354	84658
1968	FEB		0		0		0		0		6354	84658
1968	MAR		G		0		, 0		0		6354	84658
1968	APR		1920		0		1920		0		6354	84658
1968	MAY	1	1220		C		11220	0215 <u>-</u> 21 to	0		6354	84658
1968	JUN	.1	341C		0		0				33416	32089
1968	JUL	2	0190		0		0			34 C. III	13225	32039
1968	AUG	1	.3610		0		0	136			0	3165
1968	SEP		7930		0	, ep es	0	79	30 		ე 	7875
1968	TOTAL	•	0820		0		15680	551	40		,	
1968	DCT		2350	4	0		2350		٥		0	7368
1968	VCN		0		0		0		0		0	7868
1968	DEC		0		0		. 0		0	€	0	7868
1969	JAN		0		0		0		0		0	7368
1969	FEB		0		0		C		0		0	7369
1969	MAR		0		0		0		0	10	0	7868
1969	APR	55	2260		0		2260		0		0	7368
1969	MAY	1	1440		Q		5828				0	7868
1969	JUN		12940		0		0				. 0	7863
1969	JUL		9230		0		0				9024	
1969	AUG		6070		O		0				. 0	2411
1969	SEP		7930		0		0	79	30		0	2981
1040	TETAL		72240		0		10438	618	0.2			

YEAR	MONTH		ATION		ATION	+	T A In	ER	SUPPL	. Y	+ 	AEB STD	KAGE	(E 3)	m)
2		DEM	AND	SHUK	TAGE	+	NATUR	AL	STOR	RAGE	+	AMERICAN	PA	LIS	ADE:
1.0					**	+	FLOW			TER .		FALLS RE		SER	VDI:
		ACRE	FEET	ACRE	FEET	+	AC. F	Т.	AC.	FT.	+	ACRE FEE	T AC	RE	FEE'
1969	CCT	g	1790		0		179	0		0		2386			800
1969	NOV		0		0			0	,	0		2886			008
1969	DEC		O		0			0		Ü		2886			800
1970	JAN		0		0			0		0		2886 2886			800
1970	FEB		0		Ü			0		0		2886			800
1970	MAR		0		0		144			n		2886			80C
1970	APR		1440		0		991			0		2886			800
1970	MAY		9910 2770		٨		1277		#2	0		2886			800
1970	NUL		9340		0			C	19	340		27486			039
1970	JUL AUG		5280		Ö			Ö		280		11206		32	089
1970 1970	SEP	•	7290		Ō		56	57		633		10573		76	369
1970	TOTAL	6	8820		0		325	57	36	253				o quadro VII	
1970	CCT		2030		0	othe make	203	30	2	0		14073			800
1970	NOV		0		0			0		0		14073			800
1970	DEC		0		0			0	8.0	0		14073			800
1971	JAN		0		0			0		0		14073			0081
1971	FEB		, 0		0		W	0		0		14073 14073			800
1971	MAR		0		U		9.9	0		0		1407			800
1971	APR		1110		0		11: 94			0		14073			800
1971	MAY		9490		0		129			0		1407			000
1971	JUN		12900 20320		0		79		12	374		3445			2089
1971	JUL		16780		n		, ,	Ö		780		1767			2089
1971 1971	AUG Sep	•	8290		Õ			0		290		938		9 (238
1971	TCTAL		70920		0		334	76	37	444					ک جب شدن
1971	DCT		1900		0		19	00		0		1106			0800
1971	NOV		0		0			0	2	0		1106			0800
1971	DEC		0		0			0		0		1106			0800
1972			0		0			0		0		1106			0800
1972	FEB		0		0			0		0		1106			0800 0800
1972			0		0			C		0		1106 1106			0800
1972			1540		0		15			0		1106			0800
1972			10740		0		107 129		9	0		1106			0800
1972			12940	er:	0		37		3.6	290		3053			2089
1972			20000 15750		0		- (0		750		1478			2089
1972 1972			7600		0		26			947		984			0110
	TCTAL		 70470				334	 83	36	987	- 40 4				

YEAR	MENTH	IRRIGATION	IRRIGATION SHORTAGE	+ WATER	SUPPLY	A A & B STORA	GE (EDM)
		DEMAND	SURINGE	+ NATURAL	STORAGE	+ AMERICAN	PALISADE
				+ FLOW	WATER	+ FALLS RES	RESERVOI
		ACRE FEET	ACRE FEET	+ AC. FT.	AC. FT.	+ ACRE FEET	ACRE FEE
1972	тэа	1660	0	1660	0	12047	90800
1972	VON	. 0	0	0	0	12047 12047	90800 90800
1972	CEC	0	0	0	0	12047	90800
1973	JAN	0	U	. 0	0	12047	90800
1973	FEB	0	0	0	0	12047	90800
1973	MAR	1090	n	1090	û	12047	90800
1973	APR	10580	0	6354	4226	42600	90800
1973 1973	JUN	12960	ő	0	12960	29640	32089
1973	JUL	19340	0	0	19340	10300	32089
1973	AUG	15260	Ō	0	15260	٥	26471
1973	SEP	7560	O	3178	4382	0	57314
1973	TOTAL	68450	0	12282	56168		
1973	CCT	1830	0	1830	0	2002	90800
1973	NOV	0	. 0	0	0	2002	90800
1973	DEC	0	0	0	0	2002	90800
1974	JAN	0	0	0	0	2002	90800
1974	FEB	0	0	0	0	2002	90800
1974		0	0	0	0	2002 2002	90800
1974		1140	0	1140 10870	0	2002	9080
1974		10870	0	13370	Ö	2002	90806
1974		13370	0	5500	13790	33036	32089
1974		19290 15240	0	3500	15240	17796	32089
1974 1974		7800	Ö	õ	7800	9996	7185
1974	TCTAL	69540	0	32710	36830		
1974	OCT	2040	0	2040	0	15358	90809
1974		0	0	0	0	15358	9080
1974		0	0	. 0	0	15358	9080
1975		0	0	0	0	15358	9080
1975	FEB	C	0	0	0	15358	9080
1975	MAR	0	0	0	0	15358	9080
1975		510	0	510	0	15358	9080
1975		5200	0	5200	0	15358 15358	9080
1975		12760	0	12760	8808	38738	3208
1975		20270	0	12182	16390	22343	3208
1975 1975		16390 8920	0	0	8920	13423	8480
	TOTAL	66090	C	32692	33398		~~~~~
						A&B 72	

YEAR	MONTH	OF STREET, STR	NOITA	IRRIG . SHOR	ATION	÷	WATER	SUPPL	Υ	+ -+-	ARB STORA	GE (EDM)
		LEM	AND	Fanok	IAGE	+	NATURAL	STER	AGE	4	AMERICAN	PALISADE.
						+	FLDW	WAT	ER	+	FALLS RES	RESERVOI
		ACRE	FEET	ACRE	FEET	+	AC. FT.	AC.	FT.	+	ACRE FEET	ACRE FEE
1975	CCT		1980		0		1980		0		12236	77276 77276
1975	NOV		0		0		0		0		12236 12236	77276
1975	DEC		0		0		0		. 0		12236	7727 6
1976	JAN.	2.	0		0		0		۵		12236	7727 6
1976 1976	FEB Mar		0		0		o o		0		12235	7727 6
1976	APR		540		0		540	2	Ď		12236	7727 6
1976	MAY	1	.0380		Ô		10380		0		12236	7727 6
1976	JUN		3970		٥		13970		0		46826	3208 9
1976	JUL		0430		C		2643	177	787		29039	3208 9
1976	AUG		5070		0		0	150	70		13969	3208 9
1976	SEP		7390	10 10 10 10 10 10 10 10 10 10 10 10 10 1	0		4750	26	40		11330	8895 6
1976	TOTAL	6	9760	ü	0		34263	354	97			
1976	CCT		1750		0		1750	4	o		12695	9080 0
1976	NOV		0		0		0		0		12695	9080 0
1976	DEC		0		. 0		0		0		12695	9080 0
1977	NAL		0		0		0		0		12695	9080 0
1977			0		0		0		0		12695	9080 0
1977			0		0		0		0		12695	9080 0 3208 9
1977			2510		0		2510	4 :	342		46326 40484	3205 9
1977		•	9059		2717		0 C		113		32371	3208 9
1977			11589		3476 4935		C	116			20737	32089
1977			16619 13329		3998		Č		331		11406	3208 9
1977 1977		4	5490		0		3162		328		9078	32089
1977	TOTAL		0346	-1	5176		7422	37	748			
1977	GCT		1030		0		1030		0		5759	20358
1977			0		0		C		0		5759	20358
1977	CEC		0		0		0		0		5759	20358
1978	JAN		0	3	0		0		0		5759	20358
1978			0		0		0		0.		5759	20358
1973			0		0		. 0		0		5759	20355
1978			720		0		720	10 12	0		5759	20358
1978			9640		0		2643	63	997		5759	20358
1978			12810		0		12810		0		5759	20358
1978			18480		0		4761	137			26110	32089
1973			15120		0		15120		0		26110	32039 90050
1978	SEP		5280		0		5280 	~~~~	0		26110	700DU
1978	TCTAL		3080		C		42364	20	716			
											A&B 729	9